Part 2 WHY DO WE COMPACT SOIL ?



"You don't always have to do things right, but it sure helps in a pinch" - Jimmy Doolittle







John L. McAdam (1756-1836)

In 1816 Scotsman John L. McAdam published a book on road building that promoted a cambered 10-inch thick course of aggregate base rock, 16 feet wide. It employed a top course of < 2 inch rocks that each weighed less than 6 ounces, underlain by increasingly larger stones. These were then packed down by animals and wagon wheels.



- The first National Road, or Pike, was constructed westward from Cumberland, Maryland beginning in May 1811. The road corridor was 66 ft wide.
- In the 1830s the McAdam paving process was employed to construct a pavement surface 16'-10" wide, 10 inches thick, with a minimum camber of 5 inches. The grades were restricted to less than 5 degrees (8.75%).





- In 1902 Edgar Purnell Hooley from Nottinghamshire patented the process of heating tar adding slag or macadam to the mix then breaking stones within the mixture to form a smooth road surface. In 1903 he formed TarMacadam Syndicate Ltd and registered Tarmac® as a trade mark.
- The dust problem was thereby solved by spraying tar on the surface to create tar-bound macadam, while creating a much smoother riding surface.
- Tarmac was the forerunner of modern mixed asphalt pavement, often referred to as "blacktop." This method mixed the aggregates into the hot asphalt with the binding material before being laid on a compacted surface.



The vehicle that changed everything



- In 1913 the average price of an American car was \$2,635. By 1925 this figure dropped to \$870 because of mass production techniques funded by defense contracts during the First World War.
- By 1925 Ford's Model T Runabout (shown here) sold for just \$260. The sudden affordability of motor vehicles led to the public's demand for a better system of roads and highways, funded by gasoline taxes.



Importance of compaction

In the 1930s the importance of subgrade compaction, aggregate subbase, and aggregate baserock became increasingly appreciated, especially on expansive clay soils



Reducing water penetration Concrete pavements on expansive soils presented enormous challenges



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The late 1930s witnessed the introduction of impervious shoulders, asphaltic membranes, and sealed expansion joints



California Bearing Ratio (CBR) Test



Between 1927-30 O.J. Porter developed the California Bearing Ratio (CBR) and soil swell test, which measured the penetration of compacted soil to evaluate the relative stiffness of pavement subgrades and aggregate base courses, by comparing the penetration resistance of these materials with that of crushed limestone. The stated intent of the CBR test was to evaluate the load bearing capacity of the pavement subgrade.







Five-ton limestone rollers were used to compact crushed limestone and river gravels in the China-Burma-India Theater during World War II, using the principles first advanced by of John McAdam in 1816, but verifying with CBR test data





- Left: Charts showing the benefits of one-dimensional compression on soil structure, changing from a flocculated (open) to more dispersed (layered) structure
- Right: Impact of compaction on permeability of Siburua clay, illustrating the dramatic decrease in permeability with increasing density and water content. Both charts from T.W. Lambe, in Leonards' *Foundation Engineering* (1962)



Benefits of compaction

- Influence of molding water content and soil structure on swelling characteristics of sandy clay, from Seed and Chan (1959).
- Note significant increase in water content on the "dry" side of the compaction curve
- This is why it is so important to moisture condition expensive soils "wet" of optimum moisture content



We also compact soil and rock mixtures to increase their effective shear strength, making them more able to resist gross deformations

FILL PRISMS



We compact soils to reduce the long-term settlement. Fill prisms reach their greatest dimensions over old watercourses, like the one shown here. Excessive settlement may eventually lead to complete slope failure (note scarp)

DIFFERENTIAL FILL THICKNESS



We compact soil to reduce differential settlement. Sidehill embankments are of differing thickness, which promotes differential settlement and differential heave



SLOPE CREEP AND DILATION

We compact (densify) fine grained soils so they absorb less free moisture. Soil tends to absorb moisture with time and softens, promoting bearing capacity failures, settlement, loss of strength and slope creep, evidenced here by linear tension cracks in the pavement.



PRIMARY CONSOLIDATION OCCURS WHEN INTERSTITIAL PORE WATER IS SQUEEZED OUT OF SATURATED CLAY

Primary consolidation occurs when water is expelled from the pore spaces of a saturated soil. It is not usually a problem in compacted clayey embankments less than 15 to 20 feet deep.

1-D CONSOLIDATION







TRADITIONAL REPRESENTATIONS OF PRIMARY CONSOLIDATION TESTING.

Estimates of consolidation-induced settlement rely upon data derived from one dimensional odometer tests, like that sketched here.

SECONDARY CONSOLIDATION



pore water squeezed out clay particles randomly oriented

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load sustained for long period of time clay particles slowly realign themselves perpendicular to the applied stress.

Secondary consolidation occurs indefinitely as clay platelets re-align themselves under sustained loading and pore water is expelled. Usually occurs in underconsolidated estuarine and lacustrine clays.



- The time required for consolidation to occur depends on the imposed load (surcharge), the thickness of the compressible strata, and the length of the pore water drainage path(s).
- Primary consolidation ceases after a predictable period, but secondary consolidation may continue at a nearconstant rate, for a much longer period of time.



 Lab compression data from an urban fill placed in 1963 and sampled in 1985, from depths between 24 and 54 feet. <u>Secondary consolidation</u> was not evidenced in the consolidation apparatus until after a week. These one-dimensional consolidation tests were continued for 14 weeks (from Rogers, 1992).



Honeycomb structure of a clay-silt sediment

Honeycomb structure of a silty sand

Hydrocompression occurs water is added to mixtures of silt, silty sand or aeolian silts and sands, which have not previously been saturated under sustained load. The figure at upper left is taken from an article by Arthur Casagrande in the *Journal of the Boston Society of Civil Engineers* in April 1932.









Jack W. Hilf, Ph.D., PE led a group of engineers at the **US Bureau of Reclamation** that performed pioneering research on low relative density and susceptibility to hydrocompression, while working on Trenton Dam project in Nebraska.



- The Bureau of Reclamation constructed a series of distribution canals like the Meeker-**Driftwood Canal in the Republican River** Valley of Nebraska, shown above left. These were founded on low density loess and blow sands that were susceptible to densification by seepage from the canals.
- Engineering geologists with the California **Department of Water Resources made similar** discoveries a few years later in the Kern and Tulare Basins (lower left), while working on the State Water Project canal.

LONG TERM BEHAVIOR



Long term behavior is usually influenced by the aggregate sum of settlement, heave and creep of fill and underlying foundation materials





- Another form of hydrocompression is loosley termed "clod consolidation." This is actually a mechanism of *soil collapse* frequently caused by percolation of free water (in this case, through a gunite pool shell) into low density fill of low water content.
- This view shows typical settlement of concrete flatwork placed near the crest of a modest embankment. The loose-dumped muck from the pool excavation has settled about 6.5 inches (seen at far right).