

# Fundamental Mechanics of Materials Equations

## Basic definitions

Average normal stress in an axial member

$$\sigma_{avg} = \frac{F}{A}$$

Average direct shear stress

$$\tau_{avg} = \frac{V}{A_v}$$

Average bearing stress

$$\sigma_b = \frac{F}{A_b}$$

Average normal strain in an axial member

$$\epsilon_{avg} = \frac{\delta}{L} \quad \epsilon_{transverse} = \frac{\Delta d}{d} \text{ or } \frac{\Delta w}{w} \text{ or } \frac{\Delta t}{t}$$

$\gamma = \text{change in angle from } 90^\circ$

Average normal strain caused by temperature change

$$\epsilon_T = \alpha \Delta T$$

Hooke's Law (one-dimensional)

$$\sigma = E\epsilon \quad \text{and} \quad \tau = G\gamma$$

Poisson's ratio

$$\nu = -\frac{\epsilon_{lat}}{\epsilon_{long}}$$

Relationship between  $E$ ,  $G$ , and  $\nu$

$$G = \frac{E}{2(1 + \nu)}$$

Definition of allowable stress

$$\sigma_{allow} = \frac{\sigma_{failure}}{FS} \quad \text{or} \quad \tau_{allow} = \frac{\tau_{failure}}{FS}$$

Factor of safety

$$FS = \frac{\sigma_{failure}}{\sigma_{actual}} \quad \text{or} \quad FS = \frac{\tau_{failure}}{\tau_{actual}}$$

## Axial deformation

Deformation in axial members

$$\delta = \frac{FL}{AE} \quad \text{or} \quad \delta = \sum_i \frac{F_i L_i}{A_i E_i}$$

Force-temperature-deformation relationship

$$\delta = \frac{FL}{AE} + \alpha \Delta T L$$

## Torsion

Maximum torsion shear stress in a circular shaft

$$\tau_{max} = \frac{Tc}{J}$$

where the polar moment of inertia  $J$  is defined as

$$J = \frac{\pi}{2}[R^4 - r^4] = \frac{\pi}{32}[D^4 - d^4]$$

Angle of twist in a circular shaft

$$\phi = \frac{TL}{JG} \quad \text{or} \quad \phi = \sum_i \frac{T_i L_i}{J_i G_i}$$

Power transmission in a shaft

$$P = T\omega$$

$\sigma$  sigma  
 $\epsilon$  epsilon  
 $\tau$  tau  
 $\gamma$  gamma  
 $\nu$  nu  
 $\delta$   $\Delta$  delta  
 $\alpha$  alpha  
 $\phi$  phi  
 $\omega$  omega  
 $\theta$  theta

*gears*

$$r_2 T_1 = r_1 T_2$$

$$r_1 \omega_1 = r_2 \omega_2$$

*watts = Nm/s*  
*hp = 6600 in · lb/s*

## Six rules for constructing shear-force and bending-moment diagrams

Rule 1:  $\Delta V = P_0$

Rule 2:  $\Delta V = V_2 - V_1 = \int_{x_1}^{x_2} w(x) dx$

Rule 3:  $\frac{dV}{dx} = w(x)$

Rule 4:  $\Delta M = M_2 - M_1 = \int_{x_1}^{x_2} V dx$

Rule 5:  $\frac{dM}{dx} = V$

Rule 6:  $\Delta M = -M_0$

## Flexure

Flexure formula

$$\sigma_x = -\frac{My}{I} \quad \text{or} \quad \sigma_{max} = \frac{Mc}{I} = \frac{M}{S} \quad \text{where} \quad S = \frac{I}{c}$$

Unsymmetric bending of arbitrary cross sections

$$\sigma_x = \left[ \frac{I_z z - I_{yz} y}{I_y I_z - I_{yz}^2} \right] M_y + \left[ \frac{-I_y y + I_{yz} z}{I_y I_z - I_{yz}^2} \right] M_z$$

*composite beams*

$$n = \frac{E_B}{E_A}$$

Unsymmetric bending of symmetric cross sections

$$\sigma_x = \frac{M_y z}{I_y} - \frac{M_z y}{I_z} \quad \tan \beta = \frac{M_y I_z}{M_z I_y}$$

$$\sigma_A = \frac{-My}{I^T}$$

$$\sigma_B = \frac{-nMy}{I^T}$$

Horizontal shear stress associated with bending

$$\tau_H = \frac{VQ}{It} \quad \text{where} \quad Q = \sum \bar{y}_i A_i$$

Shear flow formula

$$q = \frac{VQ}{I}$$

Shear flow, fastener spacing, and fastener shear relationship

$$q_s \leq n_f V_f = n_f \tau_f A_f \quad \text{or} \quad q = \frac{V_{beam} Q}{I} = \frac{n V_{fastener}}{s}$$

For circular cross sections,

$$Q = \frac{1}{12} d^3 \quad (\text{solid sections})$$

$$Q = \frac{2}{3} [R^3 - r^3] = \frac{1}{12} [D^3 - d^3] \quad (\text{hollow sections})$$

## Beam deflections

Elastic curve relations between  $w$ ,  $V$ ,  $M$ ,  $\theta$ , and  $v$  for constant  $EI$

Deflection =  $v$

Slope =  $\frac{dv}{dx} = \theta$

Moment  $M = EI \frac{d^2 v}{dx^2}$

Shear  $V = \frac{dM}{dx} = EI \frac{d^3 v}{dx^3}$

Load  $w = \frac{dV}{dx} = EI \frac{d^4 v}{dx^4}$

# Fundamental Mechanics of Materials Equations

## Plane stress transformations

Normal and shear stresses on an arbitrary plane

$$\sigma_n = \sigma_x \cos^2 \theta + \sigma_y \sin^2 \theta + 2\tau_{xy} \sin \theta \cos \theta$$

$$\tau_{nt} = -(\sigma_x - \sigma_y) \sin \theta \cos \theta + \tau_{xy} (\cos^2 \theta - \sin^2 \theta)$$

or

$$\sigma_n = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$\sigma_t = \frac{\sigma_x + \sigma_y}{2} - \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta$$

$$\tau_{nt} = -\frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$$

Principal stress magnitudes

$$\sigma_{p1,p2} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

Orientation of principal planes

$$\tan 2\theta_p = \frac{\tau_{xy}}{(\sigma_x - \sigma_y)/2}$$

Maximum in-plane shear stress magnitude

$$\tau_{\max} = \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2} \quad \text{or} \quad \tau_{\max} = \frac{\sigma_{p1} - \sigma_{p2}}{2}$$

$$\sigma_{\text{avg}} = \frac{\sigma_x + \sigma_y}{2}$$

Absolute maximum shear stress magnitude

$$\tau_{\text{abs max}} = \frac{\sigma_{\max} - \sigma_{\min}}{2}$$

Normal, stress invariance

$$\sigma_x + \sigma_y = \sigma_n + \sigma_t = \sigma_{p1} + \sigma_{p2}$$

## Plane strain transformations

Normal and shear strain in arbitrary directions

$$\varepsilon_n = \varepsilon_x \cos^2 \theta + \varepsilon_y \sin^2 \theta + \gamma_{xy} \sin \theta \cos \theta$$

$$\gamma_{nt} = -2(\varepsilon_x - \varepsilon_y) \sin \theta \cos \theta + \gamma_{xy} (\cos^2 \theta - \sin^2 \theta)$$

or

$$\varepsilon_n = \frac{\varepsilon_x + \varepsilon_y}{2} + \frac{\varepsilon_x - \varepsilon_y}{2} \cos 2\theta + \frac{\gamma_{xy}}{2} \sin 2\theta$$

$$\varepsilon_t = \frac{\varepsilon_x + \varepsilon_y}{2} - \frac{\varepsilon_x - \varepsilon_y}{2} \cos 2\theta - \frac{\gamma_{xy}}{2} \sin 2\theta$$

$$\frac{\gamma_{nt}}{2} = -\frac{\varepsilon_x - \varepsilon_y}{2} \sin 2\theta + \frac{\gamma_{xy}}{2} \cos 2\theta$$

Principal strain magnitudes

$$\varepsilon_{p1,p2} = \frac{\varepsilon_x + \varepsilon_y}{2} \pm \sqrt{\left(\frac{\varepsilon_x - \varepsilon_y}{2}\right)^2 + \left(\frac{\gamma_{xy}}{2}\right)^2}$$

Orientation of principal strains

$$\tan 2\theta_p = \frac{\gamma_{xy}}{\varepsilon_x - \varepsilon_y}$$

Maximum in-plane shear strain

$$\frac{\gamma_{\max}}{2} = \pm \sqrt{\left(\frac{\varepsilon_x - \varepsilon_y}{2}\right)^2 + \left(\frac{\gamma_{xy}}{2}\right)^2} \quad \text{or} \quad \gamma_{\max} = \varepsilon_{p1} - \varepsilon_{p2}$$

$$\varepsilon_{\text{avg}} = \frac{\varepsilon_x + \varepsilon_y}{2}$$

Normal strain invariance

$$\varepsilon_x + \varepsilon_y = \varepsilon_n + \varepsilon_t = \varepsilon_{p1} + \varepsilon_{p2}$$

## Generalized Hooke's Law

Normal stress/normal strain relationships

$$\varepsilon_x = \frac{1}{E} [\sigma_x - \nu(\sigma_y + \sigma_z)]$$

$$\varepsilon_y = \frac{1}{E} [\sigma_y - \nu(\sigma_x + \sigma_z)]$$

$$\varepsilon_z = \frac{1}{E} [\sigma_z - \nu(\sigma_x + \sigma_y)]$$

Shear stress/shear strain relationships

$$\gamma_{xy} = \frac{1}{G} \tau_{xy} \quad \gamma_{yz} = \frac{1}{G} \tau_{yz} \quad \gamma_{zx} = \frac{1}{G} \tau_{zx}$$

where

$$G = \frac{E}{2(1 + \nu)}$$

Normal stress/normal strain relationships for plane stress

$$\varepsilon_x = \frac{1}{E} (\sigma_x - \nu\sigma_y)$$

$$\varepsilon_y = \frac{1}{E} (\sigma_y - \nu\sigma_x) \quad \text{or} \quad \sigma_x = \frac{E}{1 - \nu^2} (\varepsilon_x + \nu\varepsilon_y)$$

$$\varepsilon_z = -\frac{\nu}{E} (\sigma_x + \sigma_y) \quad \sigma_y = \frac{E}{1 - \nu^2} (\varepsilon_y + \nu\varepsilon_x)$$

Shear stress/shear strain relationships for plane stress

$$\gamma_{xy} = \frac{1}{G} \tau_{xy} \quad \text{or} \quad \tau_{xy} = G\gamma_{xy}$$

## Pressure vessels

Axial stress in spherical pressure vessel

$$\sigma_a = \frac{pr}{2t} = \frac{pd}{4t}$$

Longitudinal and hoop stresses in cylindrical pressure vessels

$$\sigma_{\text{long}} = \frac{pr}{2t} = \frac{pd}{4t} \quad \sigma_{\text{hoop}} = \frac{pr}{t} = \frac{pd}{2t}$$

$$\sigma_{\text{radial-outside}} = 0$$

$$\sigma_{\text{radial-inside}} = -p$$

## Failure theories

Mises equivalent stress for plane stress

$$\sigma_M = [\sigma_{p1}^2 - \sigma_{p1}\sigma_{p2} + \sigma_{p2}^2]^{1/2} = [\sigma_x^2 - \sigma_x\sigma_y + \sigma_y^2 + 3\tau_{xy}^2]^{1/2}$$

## Column buckling

Euler buckling load

$$P_{cr} = \frac{\pi^2 EI}{(KL)^2}$$

Euler buckling stress

$$\sigma_{cr} = \frac{\pi^2 E}{(KL/r)^2}$$

Radius of gyration

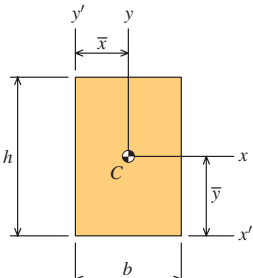
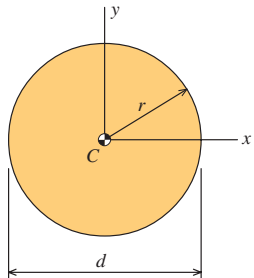
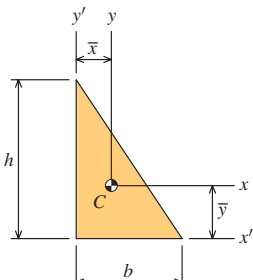
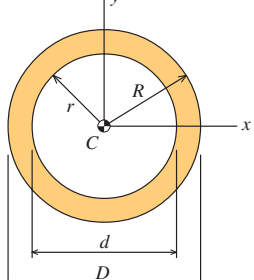
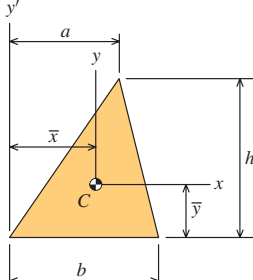
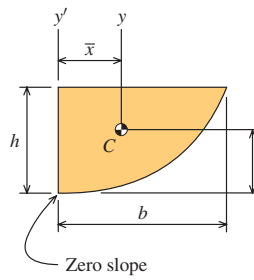
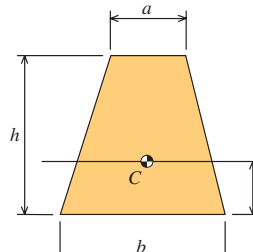
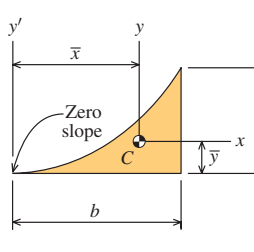
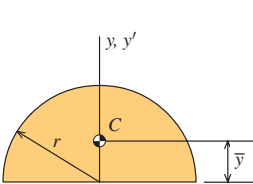
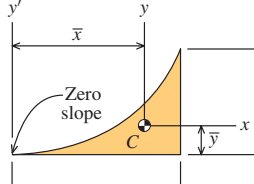
$$r^2 = \frac{I}{A}$$

$$\varepsilon_z = -\nu \left( \frac{\varepsilon_x + \varepsilon_y}{1 - \nu} \right)$$

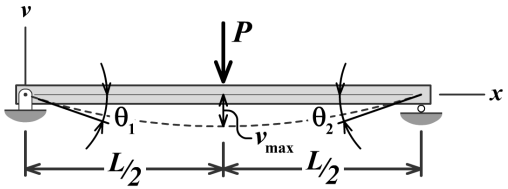
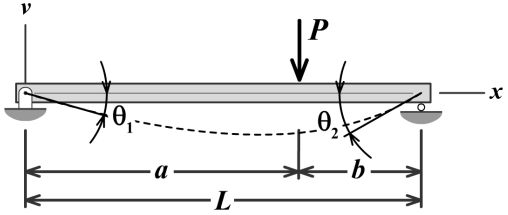
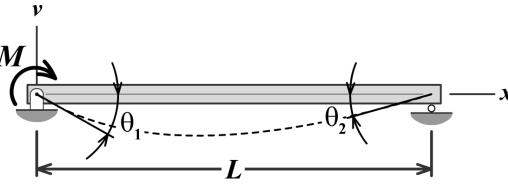
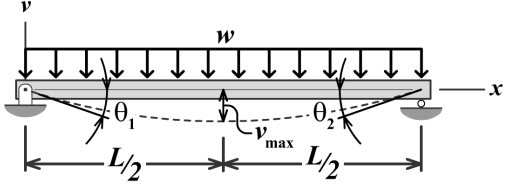
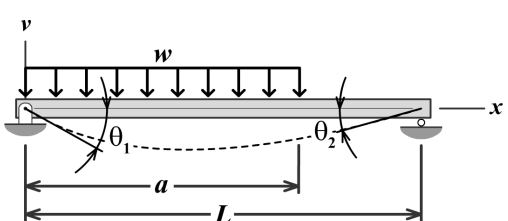
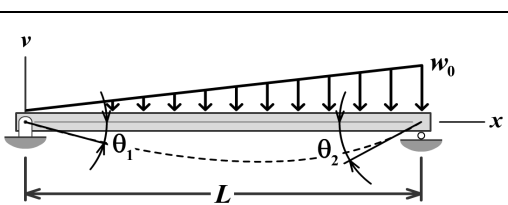
$$\bar{x} = \frac{\sum x_i A_i}{\sum A_i} \quad \bar{y} = \frac{\sum y_i A_i}{\sum A_i}$$

$$I = \sum (I_c + d^2 A)$$

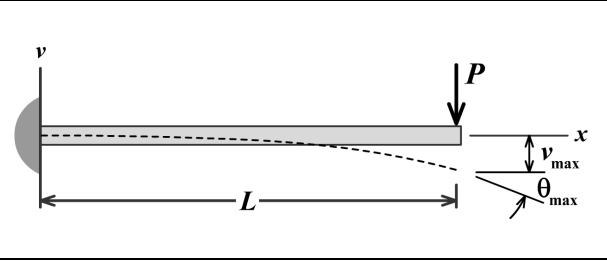
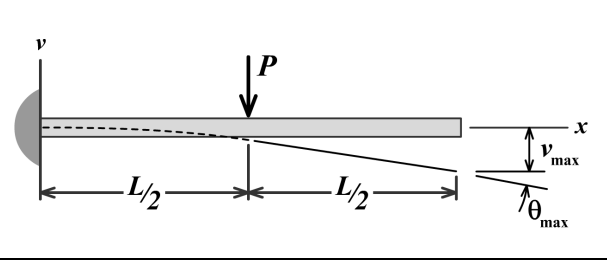
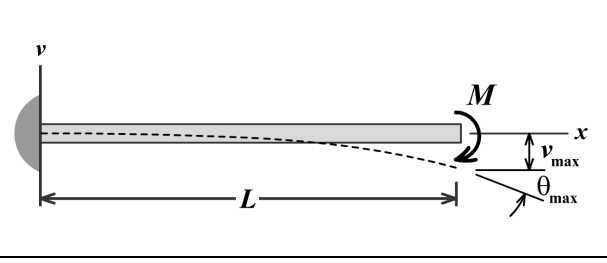
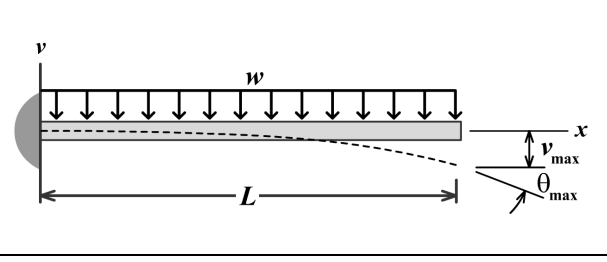
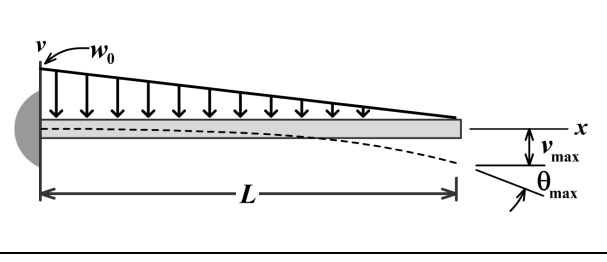
Table A.1 Properties of Plane Figures

<p><b>1. Rectangle</b></p>  $A = bh$ $\bar{y} = \frac{h}{2} \quad I_x = \frac{bh^3}{12}$ $\bar{x} = \frac{b}{2} \quad I_y = \frac{hb^3}{12}$ $I_{x'} = \frac{bh^3}{3} \quad I_{y'} = \frac{hb^3}{3}$	<p><b>6. Circle</b></p>  $A = \pi r^2 = \frac{\pi d^2}{4}$ $I_x = I_y = \frac{\pi r^4}{4} = \frac{\pi d^4}{64}$
<p><b>2. Right Triangle</b></p>  $A = \frac{bh}{2}$ $\bar{y} = \frac{h}{3} \quad I_x = \frac{bh^3}{36}$ $\bar{x} = \frac{b}{3} \quad I_y = \frac{hb^3}{36}$ $I_{x'} = \frac{bh^3}{12} \quad I_{y'} = \frac{hb^3}{12}$	<p><b>7. Hollow Circle</b></p>  $A = \pi(R^2 - r^2) = \frac{\pi}{4}(D^2 - d^2)$ $I_x = I_y = \frac{\pi}{4}(R^4 - r^4)$ $= \frac{\pi}{64}(D^4 - d^4)$
<p><b>3. Triangle</b></p>  $A = \frac{bh}{2}$ $\bar{y} = \frac{h}{3} \quad I_x = \frac{bh^3}{36}$ $\bar{x} = \frac{(a+b)}{3} \quad I_y = \frac{bh}{36}(a^2 - ab + b^2)$ $I_{x'} = \frac{bh^3}{12}$	<p><b>8. Parabola</b></p>  $y' = \frac{h}{b^2} x'^2$ $A = \frac{2bh}{3}$ $\bar{x} = \frac{3b}{8} \quad \bar{y} = \frac{3h}{5}$ <p>Zero slope</p>
<p><b>4. Trapezoid</b></p>  $A = \frac{(a+b)h}{2}$ $\bar{y} = \frac{1}{3} \left( \frac{2a+b}{a+b} \right) h$ $I_x = \frac{h^3}{36(a+b)} (a^2 + 4ab + b^2)$	<p><b>9. Parabolic Spandrel</b></p>  $y' = \frac{h}{b^2} x'^2$ $A = \frac{bh}{3}$ $\bar{x} = \frac{3b}{4} \quad \bar{y} = \frac{3h}{10}$ <p>Zero slope</p>
<p><b>5. Semicircle</b></p>  $A = \frac{\pi r^2}{2}$ $\bar{y} = \frac{4r}{3\pi} \quad I_x = \left( \frac{\pi}{8} - \frac{8}{9\pi} \right) r^4$ $I_{x'} = I_{y'} = \frac{\pi r^4}{8}$	<p><b>10. General Spandrel</b></p>  $y' = \frac{h}{b^n} x'^n$ $A = \frac{bh}{n+1}$ $\bar{x} = \frac{n+1}{n+2} b \quad \bar{y} = \frac{n+1}{4n+2} h$ <p>Zero slope</p>

## SIMPLY SUPPORTED BEAMS

Beam	Slope	Deflection	Elastic Curve
	<b>1</b> $\theta_1 = -\theta_2 = -\frac{PL^2}{16EI}$	<b>2</b> $v_{\max} = -\frac{PL^3}{48EI}$	<b>3</b> $v = -\frac{Px}{48EI}(3L^2 - 4x^2)$ <p style="text-align: right;">for <math>0 \leq x \leq L/2</math></p>
	<b>4</b> $\theta_1 = -\frac{Pb(L^2 - b^2)}{6LEI}$ $\theta_2 = +\frac{Pa(L^2 - a^2)}{6LEI}$	<b>5</b> $v = -\frac{Pa^2b^2}{3LEI}$ <p style="text-align: center;">at <math>x = a</math></p>	<b>6</b> $v = -\frac{Pbx}{6LEI}(L^2 - b^2 - x^2)$ <p style="text-align: right;">for <math>0 \leq x \leq a</math></p>
	<b>7</b> $\theta_1 = -\frac{ML}{3EI}$ $\theta_2 = +\frac{ML}{6EI}$	<b>8</b> $v_{\max} = -\frac{ML^2}{9\sqrt{3}EI}$ <p style="text-align: center;">at <math>x = L\left(1 - \frac{\sqrt{3}}{3}\right)</math></p>	<b>9</b> $v = -\frac{Mx}{6LEI}(2L^2 - 3Lx + x^2)$
	<b>10</b> $\theta_1 = -\theta_2 = -\frac{wL^3}{24EI}$	<b>11</b> $v_{\max} = -\frac{5wL^4}{384EI}$	<b>12</b> $v = -\frac{wx}{24EI}(L^3 - 2Lx^2 + x^3)$
	<b>13</b> $\theta_1 = -\frac{wa^2}{24LEI}(2L - a)^2$ $\theta_2 = +\frac{wa^2}{24LEI}(2L^2 - a^2)$	<b>14</b> $v = -\frac{wa^3}{24LEI}(4L^2 - 7aL + 3a^2)$ <p style="text-align: center;">at <math>x = a</math></p>	$v = -\frac{wx}{24LEI}(Lx^3 - 4aLx^2 + 2a^2x^2 + 4a^2L^2 - 4a^3L + a^4)$ <p style="text-align: right;">for <math>0 \leq x \leq a</math></p> $v = -\frac{wa^2}{24LEI}(2x^3 - 6Lx^2 + a^2x + 4L^2x - a^2L)$ <p style="text-align: right;">for <math>a \leq x \leq L</math></p>
	<b>16</b> $\theta_1 = -\frac{7w_0L^3}{360EI}$ $\theta_2 = +\frac{w_0L^3}{45EI}$	<b>17</b> $v_{\max} = -0.00652 \frac{w_0L^4}{EI}$ <p style="text-align: center;">at <math>x = 0.5193L</math></p>	<b>18</b> $v = -\frac{w_0x}{360LEI}(7L^4 - 10L^2x^2 + 3x^4)$

## CANTILEVER BEAMS

Beam	Slope	Deflection	Elastic Curve
	<p>19</p> $\theta_{\max} = -\frac{PL^2}{2EI}$	<p>20</p> $v_{\max} = -\frac{PL^3}{3EI}$	<p>21</p> $v = -\frac{Px^2}{6EI}(3L - x)$
	<p>22</p> $\theta_{\max} = -\frac{PL^2}{8EI}$	<p>23</p> $v_{\max} = -\frac{5PL^3}{48EI}$	<p>24</p> $v = -\frac{Px^2}{12EI}(3L - 2x) \quad \text{for } 0 \leq x \leq L/2$ $v = -\frac{PL^2}{48EI}(6x - L) \quad \text{for } L/2 \leq x \leq L$
	<p>25</p> $\theta_{\max} = -\frac{ML}{EI}$	<p>26</p> $v_{\max} = -\frac{ML^2}{2EI}$	<p>27</p> $v = -\frac{Mx^2}{2EI}$
	<p>28</p> $\theta_{\max} = -\frac{wL^3}{6EI}$	<p>29</p> $v_{\max} = -\frac{wL^4}{8EI}$	<p>30</p> $v = -\frac{wx^2}{24EI}(6L^2 - 4Lx + x^2)$
	<p>31</p> $\theta_{\max} = -\frac{w_0L^3}{24EI}$	<p>32</p> $v_{\max} = -\frac{w_0L^4}{30EI}$	<p>33</p> $v = -\frac{w_0x^2}{120LEI}(10L^3 - 10L^2x + 5Lx^2 - x^3)$