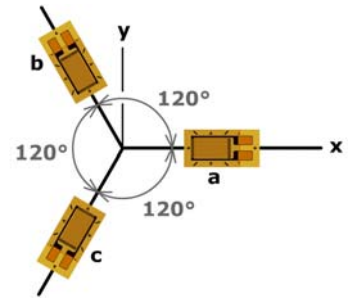
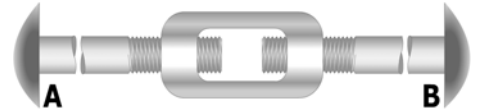


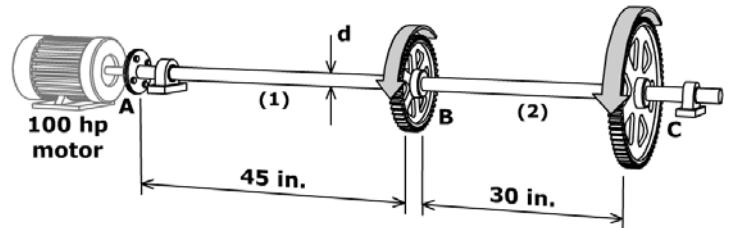
1. The following strains were measured from the strain gage rosette shown:  
 $\epsilon_a = -555 \mu\epsilon$        $\epsilon_b = +925 \mu\epsilon$        $\epsilon_c = +740 \mu\epsilon$   
 If Poisson's ratio is  $\nu = 0.3$ ,  
 (a) Determine the principal strains. [ $\epsilon_1 = 1,301 \mu\epsilon$ ,  $\epsilon_2 = -561 \mu\epsilon$ ]  
 (b) Determine the maximum shearing strain [ $\gamma_{\max} = 1,862 \mu\text{rad}$ ]  
 (c) Determine the angle from the x axis to the largest tensile principal strain. [ $\theta = 86.7^\circ \text{ cw}$ ]



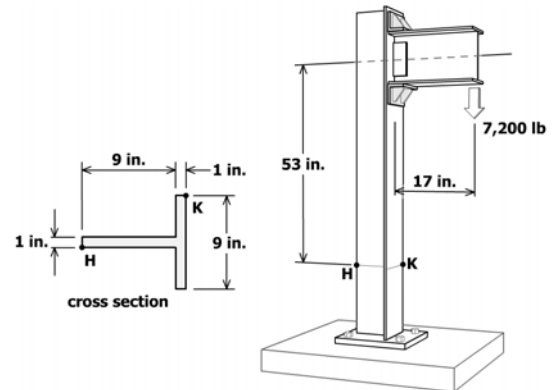
2. A steel tie rod [ $E = 200 \text{ GPa}$ ;  $\alpha = 11.9 \times 10^{-6}/^\circ\text{C}$ ] containing a rigid turnbuckle is attached to rigid walls. During the summer when the temperature was  $30^\circ\text{C}$ , the turnbuckle was tightened to produce a tension stress in the tie rod of  $15 \text{ MPa}$ . The elastic strength for the steel is  $250 \text{ MPa}$  and the ultimate strength is  $450 \text{ MPa}$ . A factor of safety of 2 is required with respect to the elastic strength, and a factor of safety of 4 is required with respect to the ultimate strength. Is the tie rod overstressed in the winter when the temperature is  $-10^\circ\text{C}$ ? Show justification for your answer. [ $\sigma = 110.2 \text{ MPa}$ ;  $FS_{\text{elastic}} = 2.27$ ;  $FS_{\text{ult}} = 4.08$ ; not overstressed]



3. A motor delivers  $100 \text{ hp}$  at  $600 \text{ rpm}$  to the end of a shaft. The gears at B and C take out  $40 \text{ hp}$  and  $60 \text{ hp}$ , respectively. Determine the minimum diameter  $d$  required for the shaft if the allowable shear stress is  $7,000 \text{ psi}$  and the angle of twist between the motor and gear C is limited to  $2^\circ$ . Assume  $G = 12 \times 10^6 \text{ psi}$ . [Note:  $1 \text{ hp} = 550 \text{ lb-ft/s}$ ]  
 $[T_1 = 875.35 \text{ lb-ft}$ ;  $T_2 = 525.21 \text{ lb-ft}$ ; based on shear stress,  $d_{\min} = 1.970 \text{ in.}$ ; based on angle of twist,  $d_{\min} = 2.003 \text{ in.}$ ; therefore,  $d_{\min} = 2.00 \text{ in.}$ ]

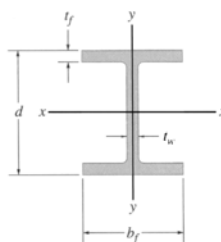


4. The tee-shaped column supports the loading shown. Determine the normal stresses at points H and K. [centroid is located  $3 \text{ in.}$  to the left of point K; moment of inertia =  $174 \text{ in}^4$ ; area =  $18 \text{ in}^2$ ; distance from centroid to  $7,200 \text{ lb}$  force is  $20 \text{ in.}$ ; moment at section of interest is  $(7,200 \text{ lb})(20 \text{ in.}) = 144,000 \text{ lb-in}$ ; axial force at section of interest is  $7,200 \text{ lb}$  (C).]  
**At point H**, bending stress is  $5,793 \text{ psi}$  (T) and axial stress is  $400 \text{ psi}$  (C); therefore, normal stress at H =  $5,393 \text{ psi}$  (T).  
**At point K**, bending stress is  $2,483 \text{ psi}$  (C) and axial stress is  $400 \text{ psi}$  (C); therefore, normal stress at K =  $2,883 \text{ psi}$  (C).]

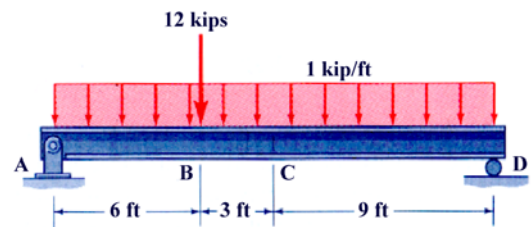


Wide-Flange Sections or W Shapes

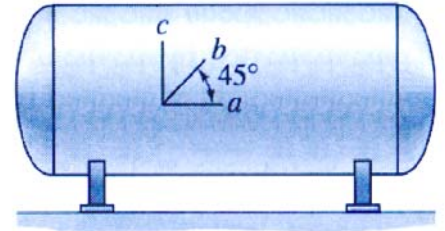
| Designation | Area<br>A       | Depth<br>d | Web<br>thickness<br>$t_w$ | Flange         |                    | x-x axis        |                 |      | y-y axis        |                 |      |
|-------------|-----------------|------------|---------------------------|----------------|--------------------|-----------------|-----------------|------|-----------------|-----------------|------|
|             |                 |            |                           | width<br>$b_f$ | thickness<br>$t_f$ | I               | S               | r    | I               | S               | r    |
| in. × lb/ft | in <sup>2</sup> | in.        | in.                       | in.            | in.                | in <sup>4</sup> | in <sup>3</sup> | in.  | in <sup>4</sup> | in <sup>3</sup> | in.  |
| W14 × 53    | 15.6            | 13.92      | 0.370                     | 8.060          | 0.660              | 541             | 77.8            | 5.89 | 57.7            | 14.3            | 1.92 |
| W14 × 43    | 12.6            | 13.66      | 0.305                     | 7.995          | 0.530              | 428             | 62.7            | 5.82 | 45.2            | 11.3            | 1.89 |
| W14 × 38    | 11.2            | 14.10      | 0.310                     | 6.770          | 0.515              | 385             | 54.6            | 5.87 | 26.7            | 7.88            | 1.55 |
| W14 × 34    | 10.0            | 13.98      | 0.285                     | 6.745          | 0.455              | 340             | 48.6            | 5.83 | 23.3            | 6.91            | 1.53 |
| W14 × 30    | 8.85            | 13.84      | 0.270                     | 6.730          | 0.385              | 291             | 42.0            | 5.73 | 19.6            | 5.82            | 1.49 |
| W14 × 26    | 7.69            | 13.91      | 0.255                     | 5.025          | 0.420              | 245             | 35.3            | 5.65 | 8.91            | 3.54            | 1.08 |
| W14 × 22    | 6.49            | 13.74      | 0.230                     | 5.000          | 0.335              | 199             | 29.0            | 5.54 | 7.00            | 2.80            | 1.04 |



5. A  $W14 \times 53$  wide-flange beam supports the distributed load and concentrated load shown. Consider location B on the beam, which is located **just to the left** of the  $12 \text{ kip}$  concentrated load. At the junction between the top flange and the beam web, determine:  
 (a) The bending stress  $\sigma$   
 (b) The shear stress  $\tau$ .  
 $[V = 11 \text{ kips}$ ;  $M = +84 \text{ kip-ft}$ ;  $I = 541 \text{ in}^4$ ;  $y = +6.30 \text{ in.}$ ;  $\sigma = -11,738 \text{ psi} = 11,738 \text{ psi}$  (C);  
 $Q = 35.2689 \text{ in}^3$ ;  $t = t_w = 0.370 \text{ in.}$ ;  $\tau = 1,938 \text{ psi}$  (downward on right face of stress element)]

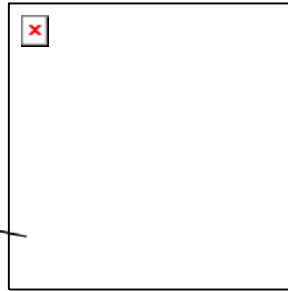
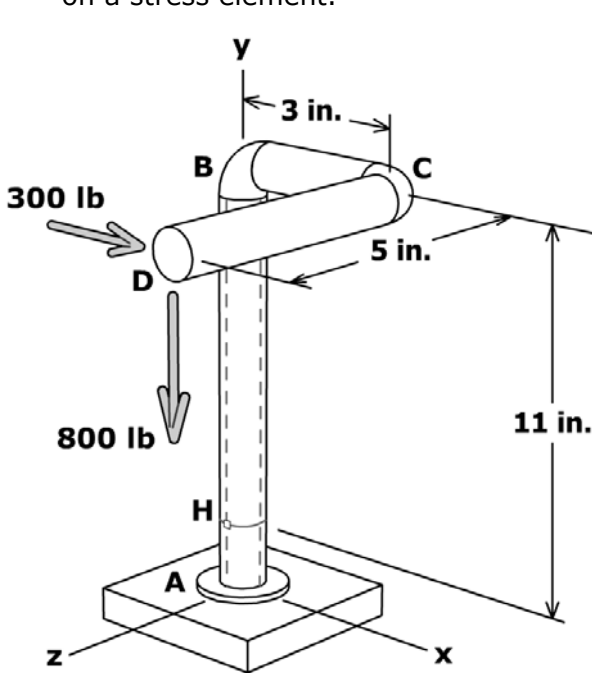


6. A 45° strain gage rosette is attached to a compressed air tank with gage "a" oriented parallel to the long axis of the tank. The tank is made of steel that has a modulus of elasticity of  $E = 29 \times 10^6$  psi and Poisson's ratio of  $\nu = 0.3$ . The inside diameter of the tank is 40 inches, the wall thickness is 0.375 inches, and the internal pressure is 180 psi. Determine the strain measured by each strain gage:  $\epsilon_a$ ,  $\epsilon_b$ , and  $\epsilon_c$ .



[ $\sigma_{long} = 4,800$  psi;  $\sigma_{hoop} = 9,600$  psi; note: shear stress is zero on planes perpendicular to the longitudinal direction and to the circumferential directions; use Generalized Hooke's Law to find  $\epsilon_a = \epsilon_x = 66.21 \mu\epsilon$  and  $\epsilon_c = \epsilon_y = 281.4 \mu\epsilon$ ; use strain transformation equation to find  $\epsilon_b = 173.79 \mu\epsilon$  at  $\theta = 45^\circ$ ]

7. For the structure shown, determine the normal and shear stresses acting at point H. Show the results on a stress element.



[At the section of interest, the forces are

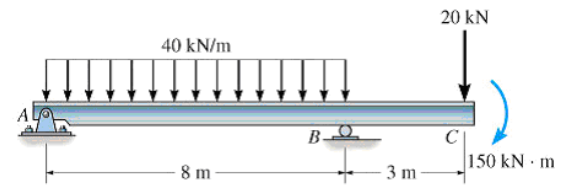
$F_x = 300$  lb,  $F_y = -800$  lb,  $F_z = 0$  lb.  
The moments are  $M_x = +4,000$  lb-in,  $M_y = +1,500$  lb-in, and  $M_z = -5,700$  lb-in.

$Q$  for the solid circular shape is  $0.28125$  in<sup>3</sup>.

**At point H**, the axial force ( $F_y = -800$  lb) causes a normal stress of 452.7 psi (C). The shear force in the  $x$  direction ( $F_x = 300$  lb) creates a shear stress of 226.4 psi, which acts in the positive  $x$  direction on the positive  $y$  face of the stress element. The torque at the section of interest ( $M_y = +1,500$  lb-in) creates a shear stress of 2,263.5 psi, which also acts in the positive  $x$  direction on the positive  $y$  face of the stress element. The bending moment about the  $x$  axis ( $M_x = +4,000$  lb-in) creates a compression normal stress at H of 17,202.9 psi (C). The bending moment about the  $z$  axis does not create a normal stress at H since H is on the neutral surface for  $M_z$ .

Altogether, the combined normal stress in the  $y$  direction at H is 12,525 psi (C) and the combined shear stress is 2,490 psi, which acts in the positive  $x$  direction on the positive  $y$  face of the stress element. There is no normal stress in the  $x$  direction at H for this structure.]

8. A W305×97 structural steel beam supports the loadings shown. The moment of inertia for the beam is  $I = 222 \times 10^6$  mm<sup>4</sup>, and the modulus of elasticity is  $E = 200$  GPa. Determine the deflection at the right end of the beam. [ $v_C = +0.5631$  mm (upward)]. To solve this problem, consider four cases: two simply supported (SS) cases and two cantilever cases.



**SS case 1:** Calculate  $\theta$  at B for 40 kN/m acting on 8 m span. Use  $\theta_B$  and 3 m overhang to calculate  $v_C$ .

**SS case 2:** Moment at B created by 20 kN load is 60 kN-m. Add the 150 kN-m concentrated moment to find that the total moment required to support the overhang is 210 kN-m. Apply a concentrated moment of 210 kN-m acting clockwise to the 8-m simple span at B and determine  $\theta_B$ . Use  $\theta_B$  and 3 m overhang to calculate  $v_C$ .

**Cantilever case 3:** Consider a 20 kN load acting at the tip of a 3-m long cantilever beam and calculate  $v_C$ .

**Cantilever case 4:** Consider a 150 kN-m moment acting clockwise at the tip of a 3-m long cantilever beam and calculate  $v_C$ .]