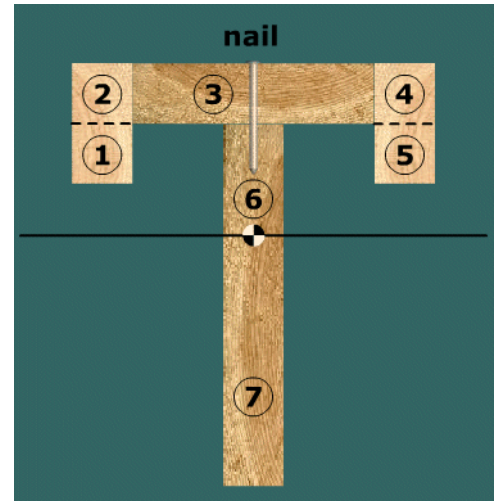


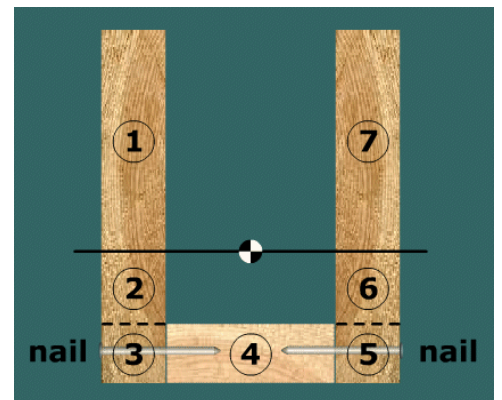
1. In calculating the shear flow associated with the nail shown, which areas should be included in the calculation of  $Q$ ? (3 points)

- \_\_\_\_\_ Areas (1) and (5)
- \_\_\_\_\_ Areas (1) through (5)
- \_\_\_\_\_ Areas (2), (3) and (4)
- \_\_\_\_\_ Areas (1), (2), (4) and (5)



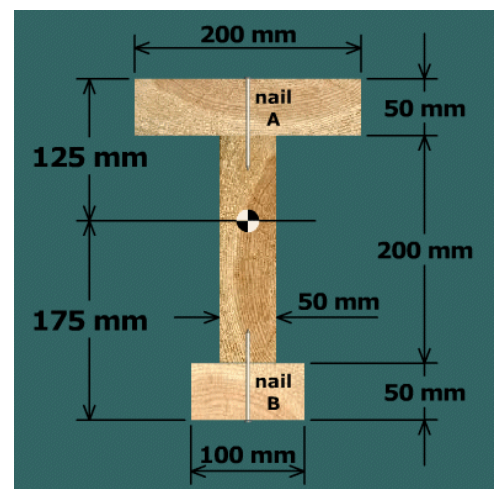
2. In calculating the shear flow associated with the two nails shown, which areas should be included in the calculation of  $Q$ ? (3 points)

- \_\_\_\_\_ Areas (2) through (6)
- \_\_\_\_\_ Areas (2), (3), (5) and (6)
- \_\_\_\_\_ Area (4)
- \_\_\_\_\_ Areas (3), (4) and (5)



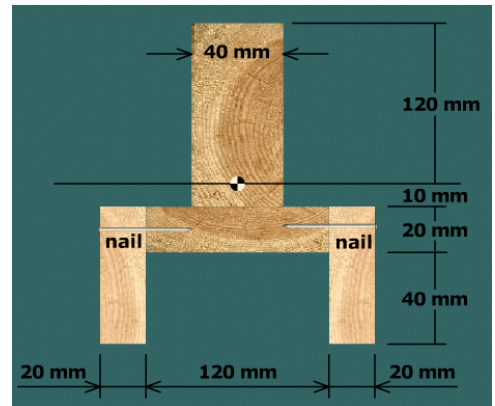
3. What is the value of  $Q$  needed to determine the shear force acting on nail  $B$ ? (6 points)

- \_\_\_\_\_  $390,625 \text{ mm}^3$
- \_\_\_\_\_  $1,140,625 \text{ mm}^3$
- \_\_\_\_\_  $875,000 \text{ mm}^3$
- \_\_\_\_\_  $750,000 \text{ mm}^3$



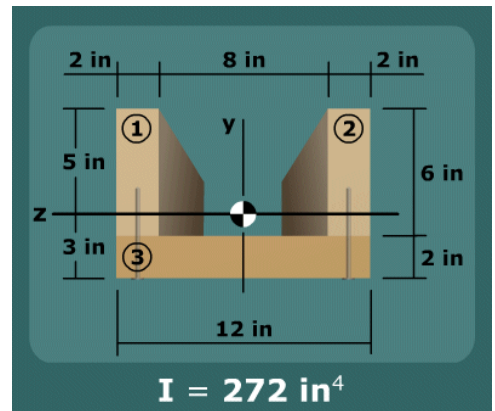
4. What is the value of  $Q$  needed to determine the shear force acting on the two nails shown? (6 points)

- \_\_\_\_\_ 480,000 mm<sup>3</sup>
- \_\_\_\_\_ 3,200 mm<sup>3</sup>
- \_\_\_\_\_ 96,000 mm<sup>3</sup>
- \_\_\_\_\_ 144,000 mm<sup>3</sup>



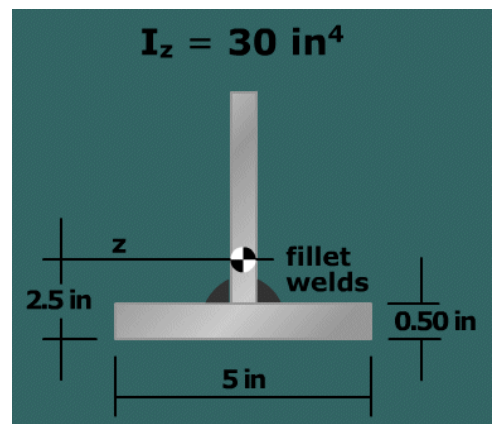
5. If boards (1) and (2) were glued to board (3) instead of nailed, what would be the shear stress in the glue if  $V = 1,125$  lb? (6 points)

- \_\_\_\_\_ 16.5 psi
- \_\_\_\_\_ 49.6 psi
- \_\_\_\_\_ 99.3 psi
- \_\_\_\_\_ 198.5 psi

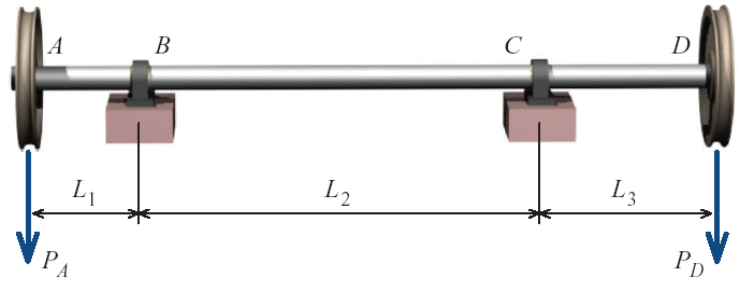


6. The allowable load for each weld is 2.4 kip/inch in the longitudinal direction. What is the maximum allowable shear force  $V$ ? (8 points)

- \_\_\_\_\_ 10.5 kips
- \_\_\_\_\_ 12.8 kips
- \_\_\_\_\_ 20.9 kips
- \_\_\_\_\_ 25.6 kips



7. A 1.00-inch-diameter solid steel shaft supports loads  $P_A = 300$  lb and  $P_D = 500$  lb. Assume  $L_1 = 5$  in.,  $L_2 = 16$  in., and  $L_3 = 8$  in. Determine the values for  $V$ ,  $Q$ ,  $I$ , and  $b$  that would be used in the maximum horizontal shear stress anywhere in the shaft. (20 points)



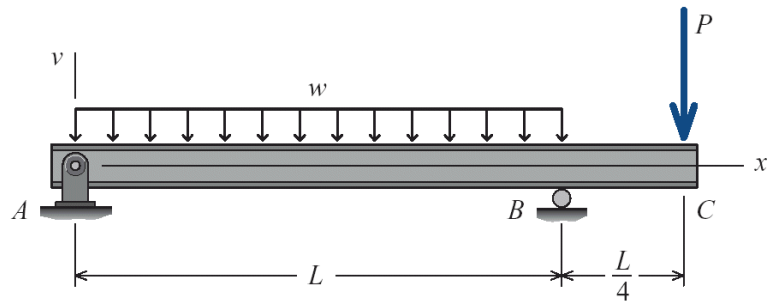
$V =$  \_\_\_\_\_ lb

$Q =$  \_\_\_\_\_ in.<sup>3</sup>

$I =$  \_\_\_\_\_ in.<sup>4</sup>

$b =$  \_\_\_\_\_ in.

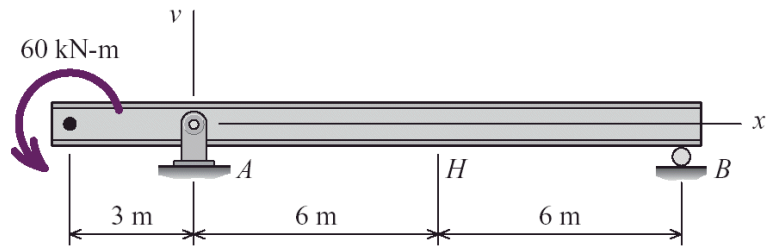
8. For the beam and loading shown, use the integration method to determine the equation of the elastic curve for segment  $AB$  of the beam. Do NOT solve for the integration constants. (8 points)



9. List the boundary, continuity, and/or symmetry conditions that could be used to solve the integration constants in the previous problem. (6 points)

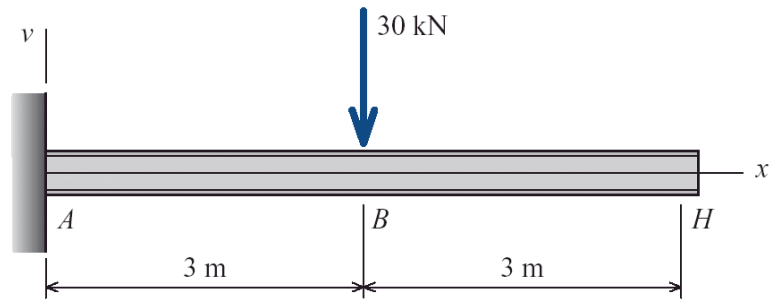
10. Determine the beam deflection at point  $H$ . Assume that  $EI = 40,000 \text{ kN}\cdot\text{m}^2$  is constant for the beam. (6 points)

$v_H = \underline{\hspace{2cm}} \text{ mm}$



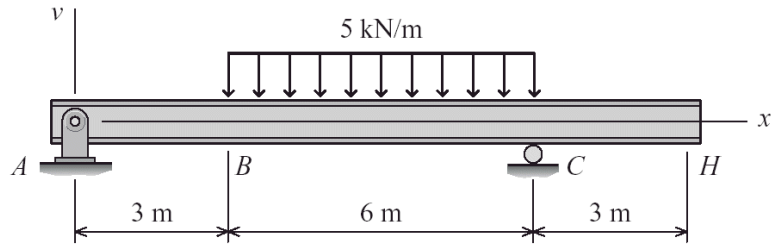
11. Determine the beam deflection at point  $H$ . Assume that  $EI = 40,000 \text{ kN}\cdot\text{m}^2$  is constant for the beam. (10 points)

$v_H = \underline{\hspace{2cm}} \text{ mm}$



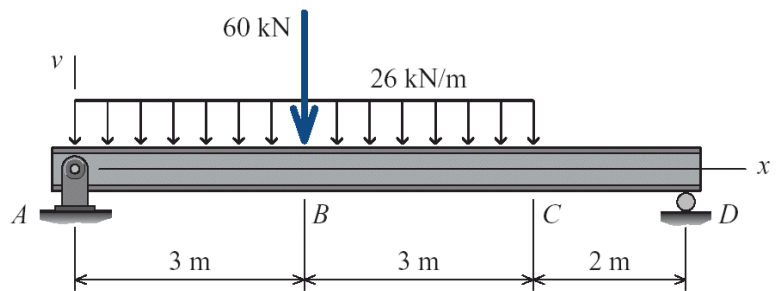
12. Determine the beam deflection at point  $H$ . Assume that  $EI = 40,000 \text{ kN}\cdot\text{m}^2$  is constant for the beam. (8 points)

$v_H = \underline{\hspace{2cm}} \text{ mm}$

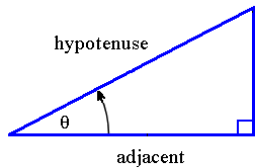


13. Determine the beam deflection at point  $C$ . Assume that  $EI = 40,000 \text{ kN}\cdot\text{m}^2$  is constant for the beam. (10 points)

$v_C = \underline{\hspace{2cm}} \text{ mm}$



# TRIGONOMETRY



$$\sin \theta = \text{opp} / \text{hyp}$$

$$\cos \theta = \text{adj} / \text{hyp}$$

$$\tan \theta = \text{opp} / \text{adj}$$

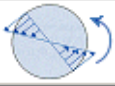


# STATICS

Symbol	Meaning	Equation	Units
$\bar{x}, \bar{y}, \bar{z}$	centroid position	$\bar{y} = \sum \bar{y}_i A_i / \sum A_i$	in, m
I	moment of inertia	$I = \sum (I_i + d_i^2 A_i)$	in <sup>4</sup> , m <sup>4</sup>
J	polar moment of inertia	J <sub>solid circular shaft</sub> = $\pi d^4 / 32$ J <sub>hollow circular shaft</sub> = $\pi (d_o^4 - d_i^4) / 32$	in <sup>4</sup> , m <sup>4</sup>
N	normal force		lb, N
V	shear force	$V = \int -w(x) dx$	lb, N
M	bending moment	$M = \int V(x) dx$	in-lb, Nm
equilibrium		$\sum F = 0$ $\sum M_{(\text{any point})} = 0$	lb, N in-lb, Nm

SECOND MOMENTS OF PLANE AREAS			
	$A = bh$	$I_x = \frac{bh^3}{12}$ $I_y = \frac{hb^3}{12}$ $I_{xy} = 0$	$I_{x'} = \frac{bh^3}{3}$ $I_{y'} = \frac{hb^3}{3}$ $I_{x'y'} = \frac{b^2h^2}{4}$
	$A = \frac{1}{2}bh$	$I_x = \frac{bh^3}{36}$ $I_y = \frac{hb^3}{36}$ $I_{xy} = \frac{b^2h^2}{72}$	$I_{x'} = \frac{bh^3}{12}$ $I_{y'} = \frac{hb^3}{4}$ $I_{x'y'} = \frac{b^2h^2}{8}$
	$A = \pi R^2$	$I_x = \frac{\pi R^4}{4}$ $I_y = \frac{\pi R^4}{4}$ $I_{xy} = 0$	$I_{x'} = \frac{5\pi R^4}{4}$
	$A = \frac{1}{2}\pi R^2$	$I_x = \frac{\pi R^4}{8} - \frac{8R^4}{9\pi}$ $I_y = \frac{\pi R^4}{8}$ $I_{xy} = 0$	$I_{x'} = \frac{\pi R^4}{8}$ $I_{x'y'} = \frac{2R^4}{3}$
	$A = \frac{1}{4}\pi R^2$	$I_x = \frac{\pi R^4}{16} - \frac{4R^4}{9\pi}$ $I_y = \frac{\pi R^4}{16}$ $I_{xy} = \frac{(9\pi - 32)R^4}{72\pi}$	$I_{x'} = \frac{\pi R^4}{16}$ $I_{y'} = \frac{\pi R^4}{16}$ $I_{x'y'} = \frac{R^4}{8}$

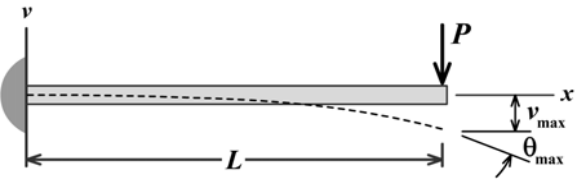
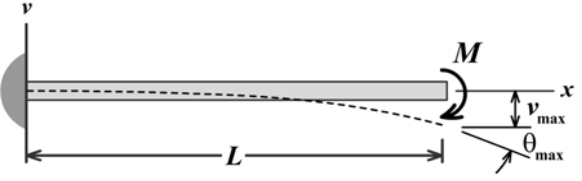
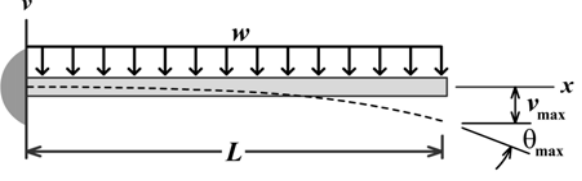
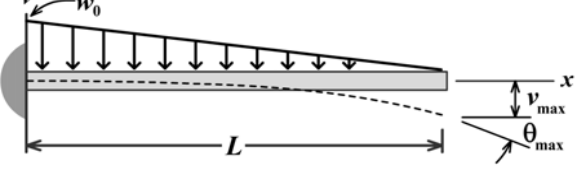
# MECHANICS OF MATERIALS

Topic	Symbol	Meaning	Equation	Units
axial	$\sigma$ , sigma	normal stress	$\sigma_{\text{axial}} = N/A$ $\tau_{\text{cutting}} = V/A$ $\sigma_{\text{bearing}} = F_b/A_b$	psi, Pa
	$\epsilon$ , epsilon	normal strain	$\epsilon_{\text{axial}} = \Delta L/L_o = \delta/L_o$ $\epsilon_{\text{transverse}} = \Delta d/d$	in/in, m/m
	$\gamma$ , gamma	shear strain	$\gamma = \text{change in angle}, \gamma = c\theta$	rad
	E	Young's modulus, modulus of elasticity	$\sigma = E\epsilon$ (one-dimensional only)	psi, Pa
	G	shear modulus, modulus of rigidity	$G = \tau/\gamma = E / 2(1+\nu)$	psi, Pa
	$\nu$ , nu	Poisson's ratio	$\nu = -\epsilon'/\epsilon$	
	$\delta$ , delta	deformation, elongation, deflection	$\delta = NL_o/EA + \alpha\Delta TL_o$	in, m
	$\alpha$ , alpha	coefficient of thermal expansion (CTE)		in/inF, m/mC
	F.S.	factor of safety	F.S. = actual strength / design strength	

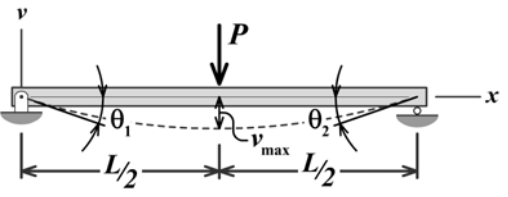
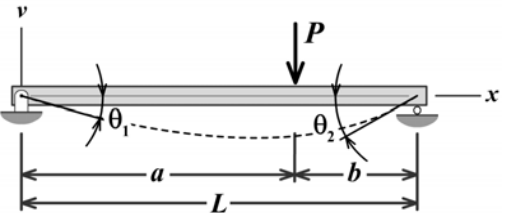
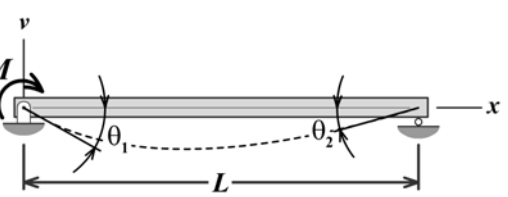
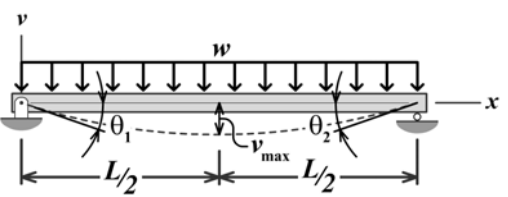
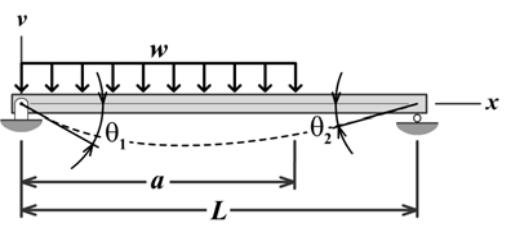
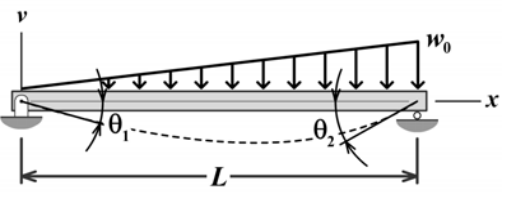
Topic	Symbol	Meaning	Equation	Units	
torsion	$\tau$ , tau	shear stress 	$\tau_{\text{torsion}} = Tc/J$	psi, Pa	
	$\phi$ , phi	angle of twist	$\phi = TL/GJ$	rad, degrees	
	$\theta$ , theta	angle of twist per unit length, rate of twist	$\theta = \phi / L$	rad/in, rad/m	
	P	power	$P = T\omega$	$r_2 T_1 = r_1 T_2$	watts = Nm/s hp=6600 in-lb/s
	$\omega$ , omega	angular speed, speed of rotation		$r_1 \omega_1 = r_2 \omega_2$	rad/s
	f	frequency	$\omega = 2\pi f$	Hz = rev/s	
	K	stress concentration factor	$\tau_{\text{max}} = KTc/J$	psi, Pa	
flexure	$\sigma$ , sigma	normal stress 	$\sigma_{\text{beam}} = -My/I$	psi, Pa	
	$\sigma$ , sigma	composite beams, $n = E_B/E_A$	$\sigma_A = -My / I^T$ $\sigma_B = -nMy / I^T$	psi, Pa	
	$\tau$ , tau	shear stress 	$\tau_{\text{beam}} = VQ/Ib$ where $Q = \sum(y_{\text{bar } i} A_i)$	psi, Pa	
	q	shear flow	$q = V_{\text{beam}} Q/I = nV_{\text{fastener}}/s$		
	v or y	beam deflection	$v = \iint M(x) dx^2 / EI$	in, m	
Topic	Equations			Units	
stress transformation	<i>planar rotations</i> $\sigma_u = (\sigma_x + \sigma_y)/2 + (\sigma_x - \sigma_y)/2 \cos(2\theta) + \tau_{xy} \sin(2\theta)$ $\sigma_v = (\sigma_x + \sigma_y)/2 - (\sigma_x - \sigma_y)/2 \cos(2\theta) - \tau_{xy} \sin(2\theta)$ $\tau_{uv} = -(\sigma_x - \sigma_y)/2 \sin(2\theta) + \tau_{xy} \cos(2\theta)$		<i>principals and max in-plane shear</i> $\tan(2\theta_p) = 2\tau_{xy} / (\sigma_x - \sigma_y)$ , $\theta_s = \theta_p \pm 45^\circ$ $\sigma_{1,2} = (\sigma_x + \sigma_y)/2 \pm \text{sqrt} \{ [(\sigma_x - \sigma_y)/2]^2 + \tau_{xy}^2 \}$ $\tau_{\text{max}} = \text{sqrt} \{ [(\sigma_x - \sigma_y)/2]^2 + \tau_{xy}^2 \} = (\sigma_1 - \sigma_2)/2$ $\sigma_{\text{avg}} = (\sigma_x + \sigma_y)/2 = (\sigma_1 + \sigma_2)/2$	psi, Pa	
strain transformation	<i>planar rotations</i> $\epsilon_u = (\epsilon_x + \epsilon_y)/2 + (\epsilon_x - \epsilon_y)/2 \cos(2\theta) + \gamma_{xy}/2 \sin(2\theta)$ $\epsilon_v = (\epsilon_x + \epsilon_y)/2 - (\epsilon_x - \epsilon_y)/2 \cos(2\theta) - \gamma_{xy}/2 \sin(2\theta)$ $\gamma_{uv}/2 = -(\epsilon_x - \epsilon_y)/2 \sin(2\theta) + \gamma_{xy}/2 \cos(2\theta)$ $\epsilon_z = -v(\epsilon_x + \epsilon_y) / (1-v)$		<i>principals and max in-plane shear</i> $\tan(2\theta_p) = \gamma_{xy} / (\epsilon_x - \epsilon_y)$ , $\theta_s = \theta_p \pm 45^\circ$ $\epsilon_{1,2} = (\epsilon_x + \epsilon_y)/2 \pm \text{sqrt} \{ [(\epsilon_x - \epsilon_y)/2]^2 + (\gamma_{xy}/2)^2 \}$ $\gamma_{\text{max}}/2 = \text{sqrt} \{ [(\epsilon_x - \epsilon_y)/2]^2 + (\gamma_{xy}/2)^2 \}$ $\epsilon_{\text{avg}} = (\epsilon_x + \epsilon_y)/2$	psi, Pa in/in, m/m	
Hooke's law	<i>1D strain to stress</i> $\sigma = E\epsilon$ <i>2D strain to stress</i> $\sigma_x = E(\epsilon_x + v\epsilon_y) / (1-v^2)$ $\sigma_y = E(\epsilon_y + v\epsilon_x) / (1-v^2)$ $\tau_{xy} = G\gamma_{xy} = E\gamma_{xy} / 2(1+v)$		<i>2D stress to strain</i> $\epsilon_x = (\sigma_x - v\sigma_y) / E$ $\epsilon_y = (\sigma_y - v\sigma_x) / E$ $\epsilon_z = -v(\epsilon_x + \epsilon_y) / (1-v)$ $\gamma_{xy} = \tau_{xy}/G = 2(1+v)\tau_{xy} / E$	psi, Pa in/in, m/m	
pressure	$\sigma_{\text{spherical}} = pr/2t$ $\sigma_{\text{cylindrical, hoop}} = pr/t$ $\sigma_{\text{cylindrical, axial}} = pr/2t$		$\sigma_{\text{radial, outside}} = 0$ $\sigma_{\text{radial, inside}} = -p$	psi, Pa	
failure theories	<i>maximum principal stress theory</i> $\sigma_{1,2} < \sigma_{yp}$		<i>maximum shear stress theory</i> $\tau_{\text{max}} < 0.5 \sigma_{yp}$	psi, Pa	



## CANTILEVER BEAMS

Beam	Slope	Deflection	Elastic Curve
	$\theta_{\max} = -\frac{PL^2}{2EI}$	$v_{\max} = -\frac{PL^3}{3EI}$	$v = -\frac{Px^2}{6EI}(3L - x)$
	$\theta_{\max} = -\frac{ML}{EI}$	$v_{\max} = -\frac{ML^2}{2EI}$	$v = -\frac{Mx^2}{2EI}$
	$\theta_{\max} = -\frac{wL^3}{6EI}$	$v_{\max} = -\frac{wL^4}{8EI}$	$v = -\frac{wx^2}{24EI}(6L^2 - 4Lx + x^2)$
	$\theta_{\max} = -\frac{w_0L^3}{24EI}$	$v_{\max} = -\frac{w_0L^4}{30EI}$	$v = -\frac{w_0x^2}{120EI}(10L^3 - 10L^2x + 5Lx^2 - x^3)$

## SIMPLY SUPPORTED BEAMS

Beam	Slope	Deflection	Elastic Curve
	$\theta_1 = -\theta_2 = -\frac{PL^2}{16EI}$	$v_{\max} = -\frac{PL^3}{48EI}$	$v = -\frac{Px}{48EI}(3L^2 - 4x^2)$ <i>for</i> $0 \leq x \leq L/2$
	$\theta_1 = -\frac{Pb(L^2 - b^2)}{6LEI}$ $\theta_2 = +\frac{Pa(L^2 - a^2)}{6LEI}$	$v _{x=a} = -\frac{Pba}{6LEI}(L^2 - b^2 - a^2)$	$v = -\frac{Pbx}{6LEI}(L^2 - b^2 - x^2)$ <i>for</i> $0 \leq x \leq a$
	$\theta_1 = -\frac{ML}{3EI}$ $\theta_2 = +\frac{ML}{6EI}$	$v_{\max} = -\frac{ML^2}{9\sqrt{3}EI}$ <i>@</i> $x = L\left(1 - \frac{\sqrt{3}}{3}\right)$	$v = -\frac{Mx}{6LEI}(2L^2 - 3Lx + x^2)$
	$\theta_1 = -\theta_2 = -\frac{wL^3}{24EI}$	$v_{\max} = -\frac{5wL^4}{384EI}$	$v = -\frac{wx}{24EI}(L^3 - 2Lx^2 + x^3)$
	$\theta_1 = -\frac{wa^2}{24LEI}(2L - a)^2$ $\theta_2 = +\frac{wa^2}{24LEI}(2L^2 - a^2)$	$v _{x=a} = -\frac{wa^3}{24LEI}(3a^2 - 7aL + 4L^2)$	$v = -\frac{wx}{24LEI}(a^4 - 4a^3L + 4a^2L^2 + 2a^2x^2 - 4aLx^2 + Lx^3)$ <i>for</i> $0 \leq x \leq a$ $v = -\frac{wa^2}{24LEI}(-a^2L + 4L^2x + a^2x - 6Lx^2 + 2x^3)$ <i>for</i> $a \leq x \leq L$
	$\theta_1 = -\frac{7w_0L^3}{360EI}$ $\theta_2 = +\frac{w_0L^3}{45EI}$	$v_{\max} = -0.00652 \frac{w_0L^4}{EI}$ <i>@</i> $x = 0.5193L$	$v = -\frac{w_0x}{360LEI}(7L^4 - 10L^2x^2 + 3x^4)$