



Update on Pilot Program to Assess Seismic Hazards in the St. Louis Metro Area

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**Earthquakes
Mean Business
February 6, 2009**

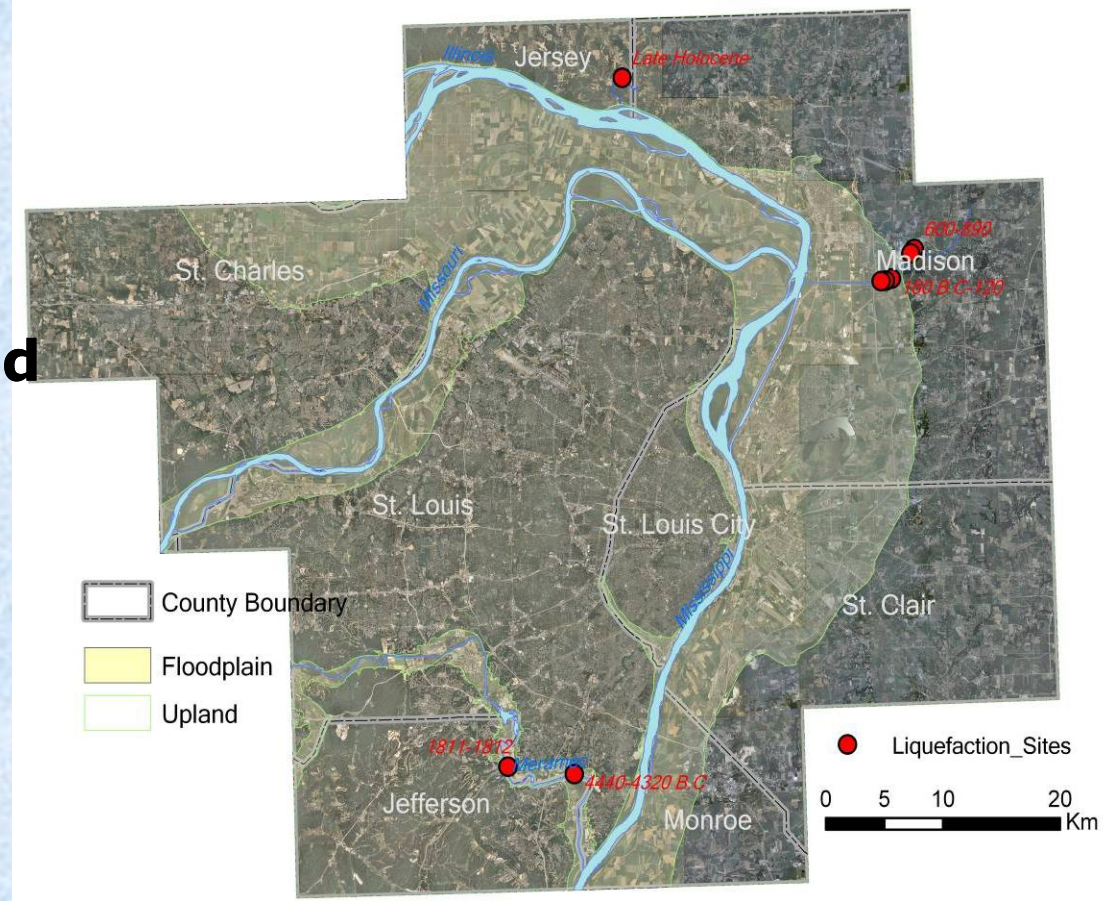
Step 1

Construction of a Virtual Geotechnical Database for the Geology Underlying the St. Louis Metropolitan Area

The St Louis study area consists of 29 USGS 7.5 minute Quadrangles in *Missouri* and *Illinois*, encompassing 4,482 sq km land area

The area consists of: **floodplains** along the rivers; and loess-covered elevated **uplands** on either side.

Earthquake liquefaction features have been identified along the major river channels; some are interpreted as having formed in **1811-1812**.



Seven GIS Geodata layers underlying the *St. Louis Metro Area*

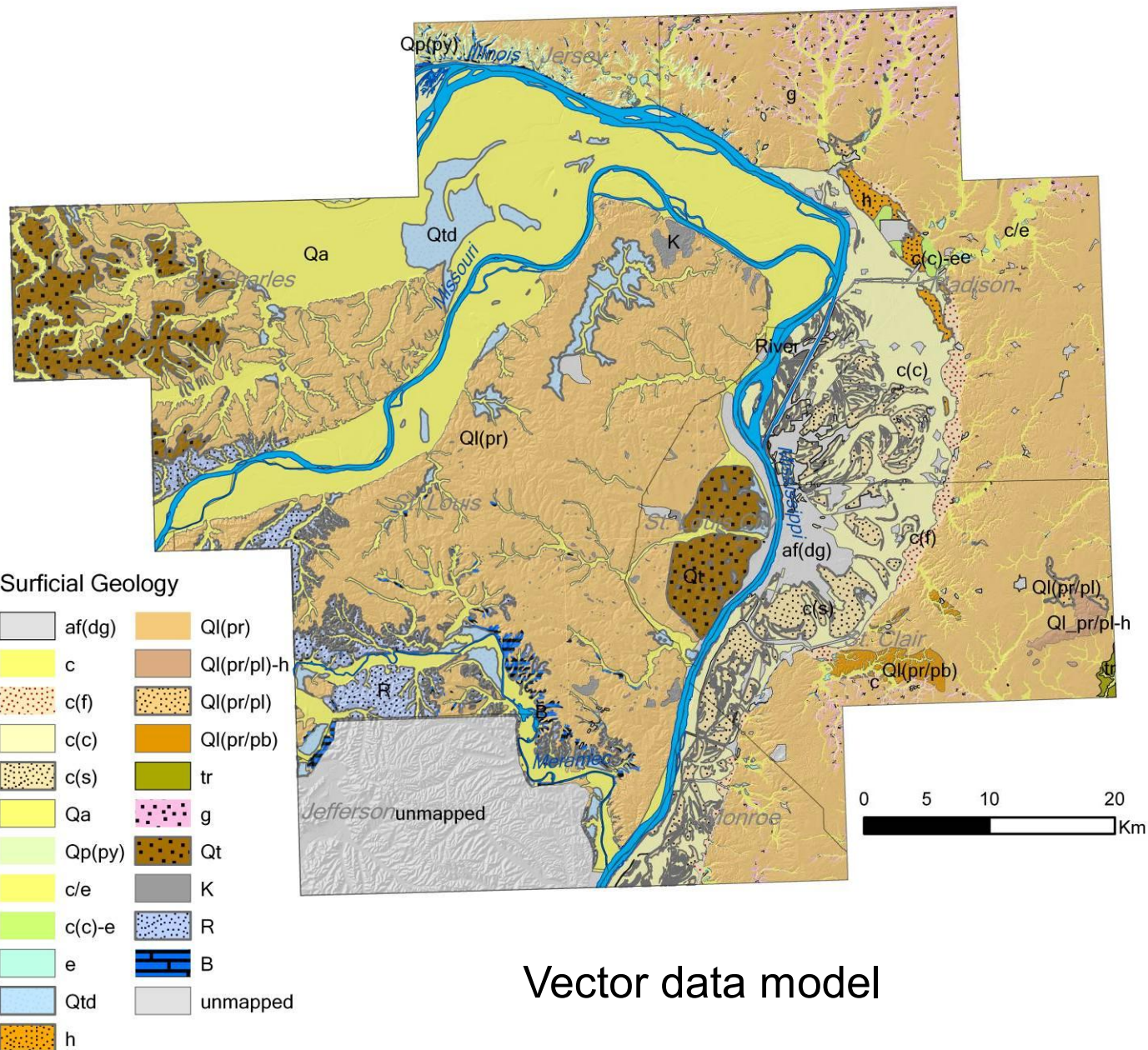
We collected and/or estimated the following information:

- 1) Surficial geology
- 2) Loess thickness
- 3) Bedrock geology
- 4) Borehole information
- 5) Shear wave velocities of surficial materials
- 6) Depth to groundwater
- 7) Depth to Paleozoic age bedrock

Goal is to estimate the severity of shaking:

- Amplification of incoming seismic energy due to soil cap overlying dense Paleozoic age bedrock
- Magnification of incoming seismic energy due to impedance contrast with the soil cap

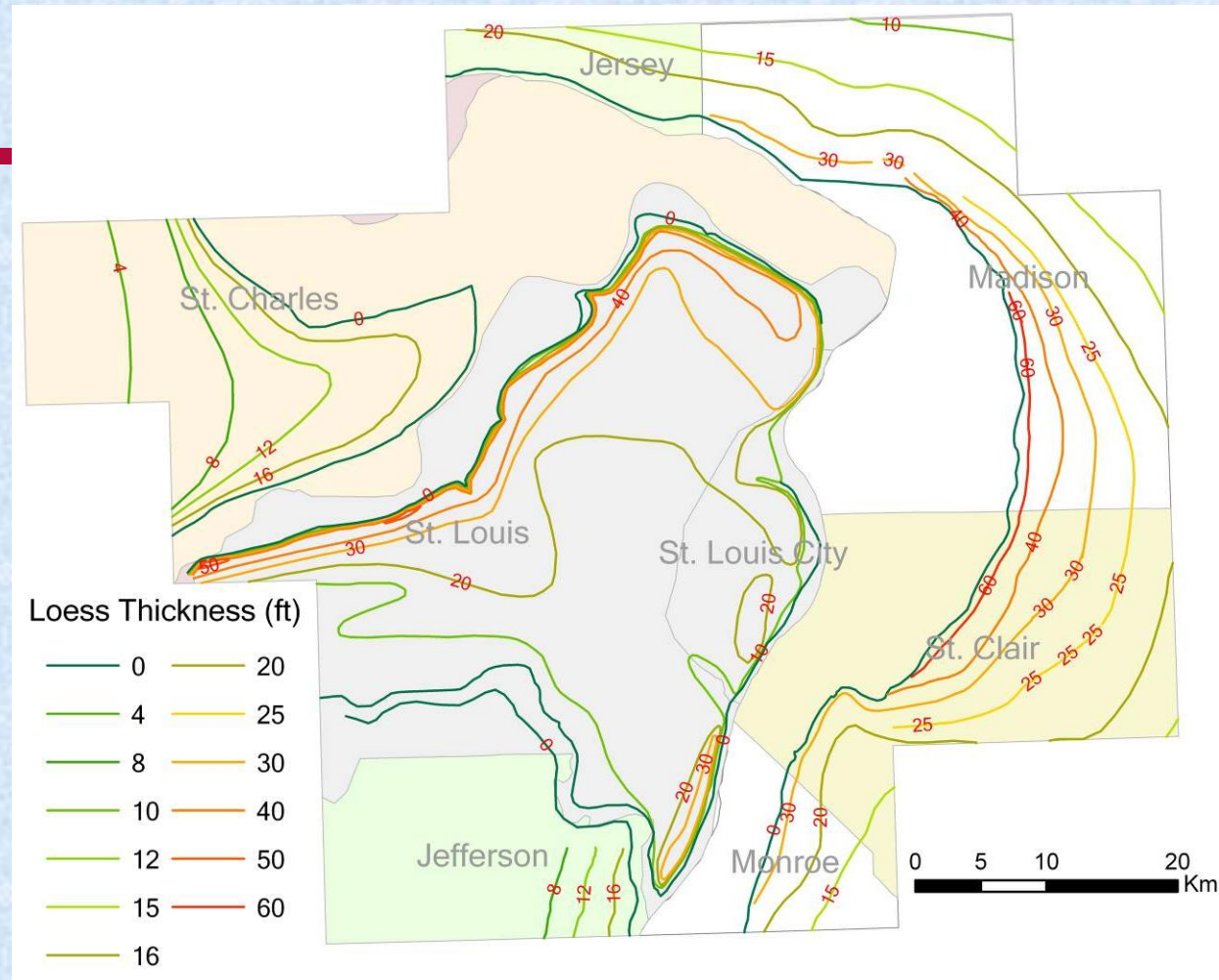
Compiled Surficial Geologic Map



Vector data model

Loess Thickness Map (in feet)

- **Loess (Peoria and Roxana Silts):**
- **Thickest along the river bluffs bordering the Missouri and Mississippi Rivers; and**
- **Thins exponentially, away from the river bluffs**

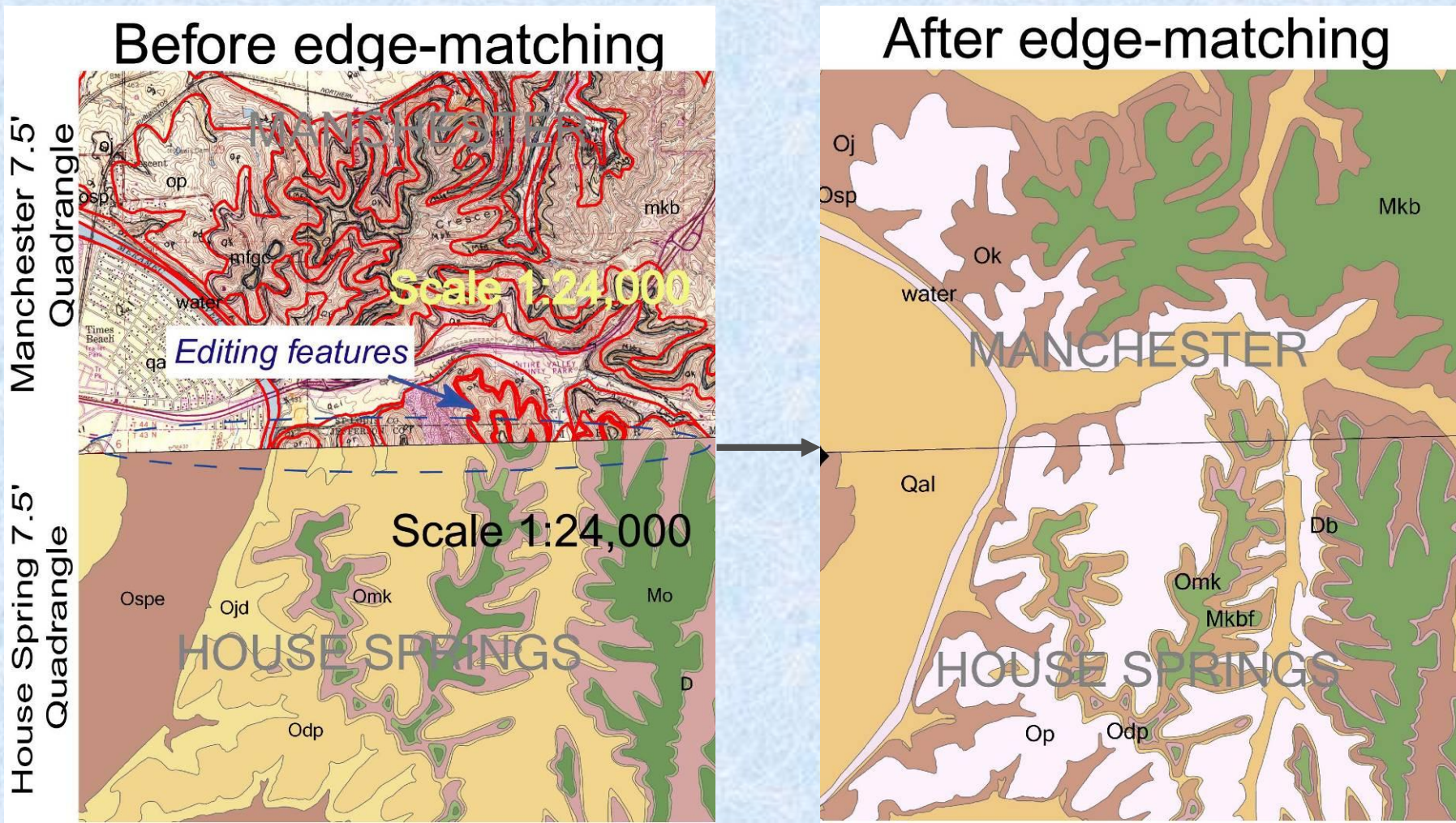


- Vector data model

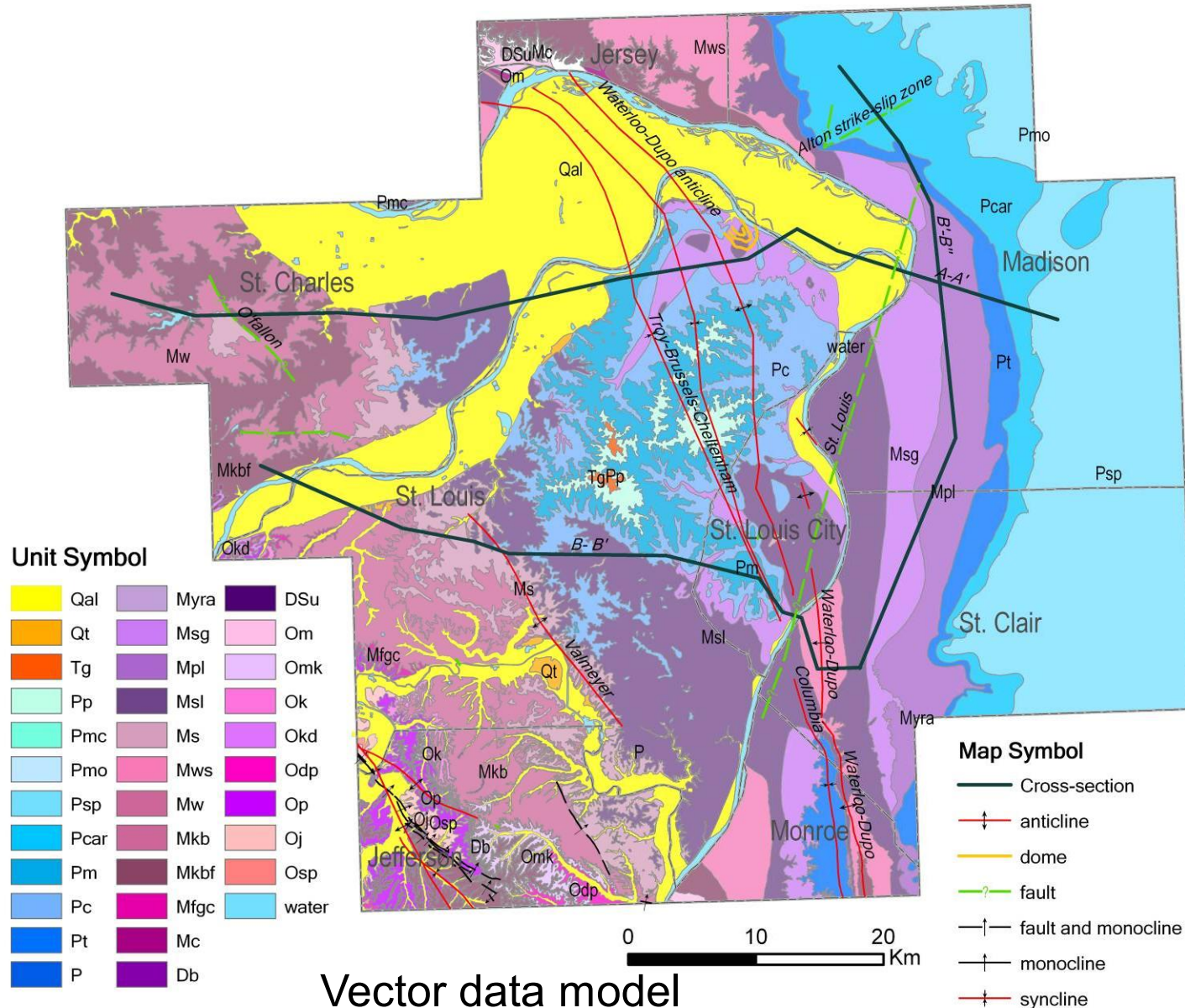
Map Scale Matching Problems

Possible Solutions:

For mismatching boundary area, editing another 24K map boundaries instead of 100K map

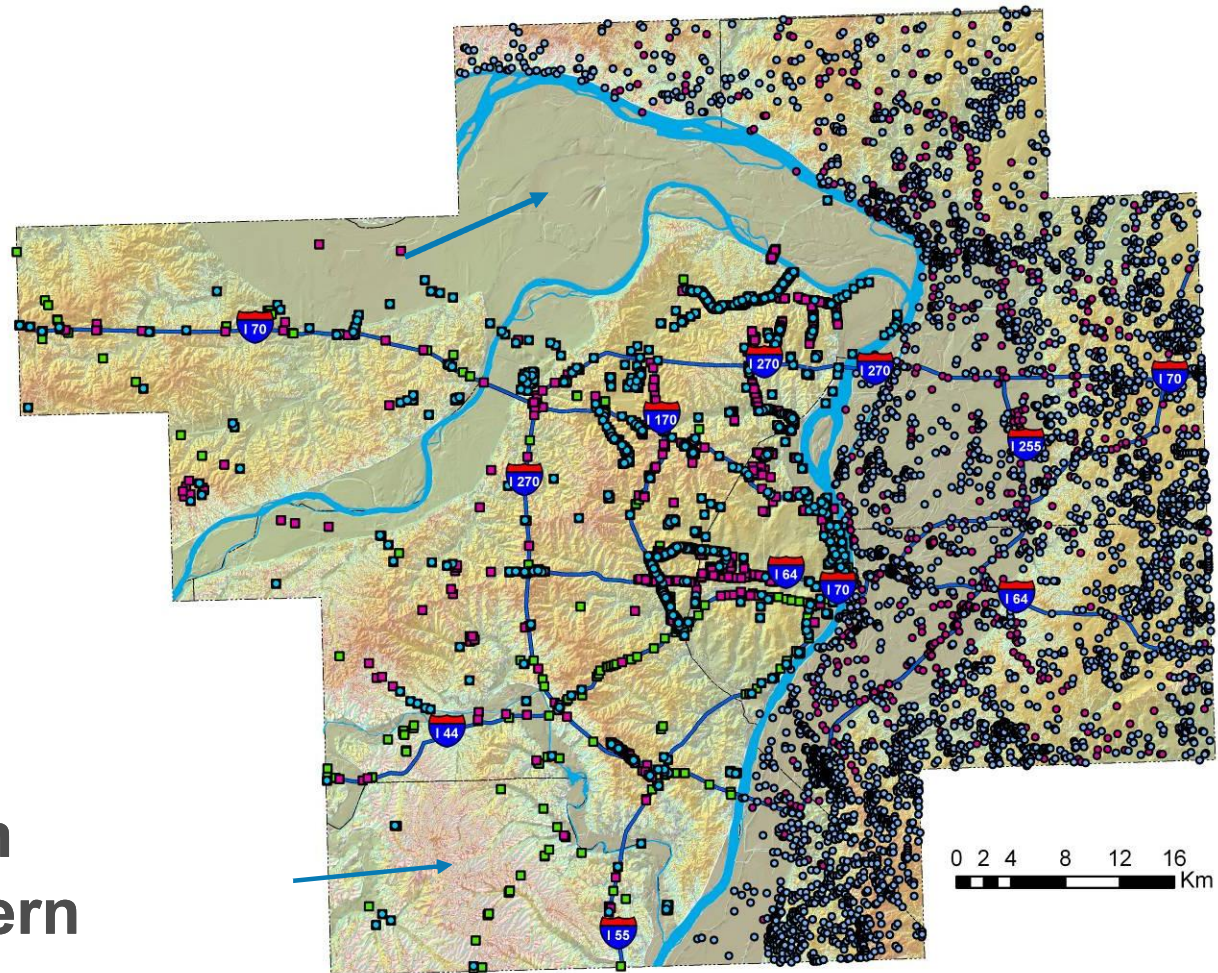


Compiled Bedrock Geology Map



Borehole Locations

- Data Sources:
 - MoDNR-DGLS
 - ISGS
- Note Data Gaps in Jefferson and eastern St. Charles counties



Geotechnical boring(MoDGLS)

Borehole Type

- Bedrock depth and type
- Corelog(RQD)
- Grain Size
- Material
- Physical property
- Water observation

Geotechnical boring(ISGS)

Borehole Type

- Highway log
- Highway/Engineering
- Highwayhead
- Log
- Water well

Vector data model

Borehole Information

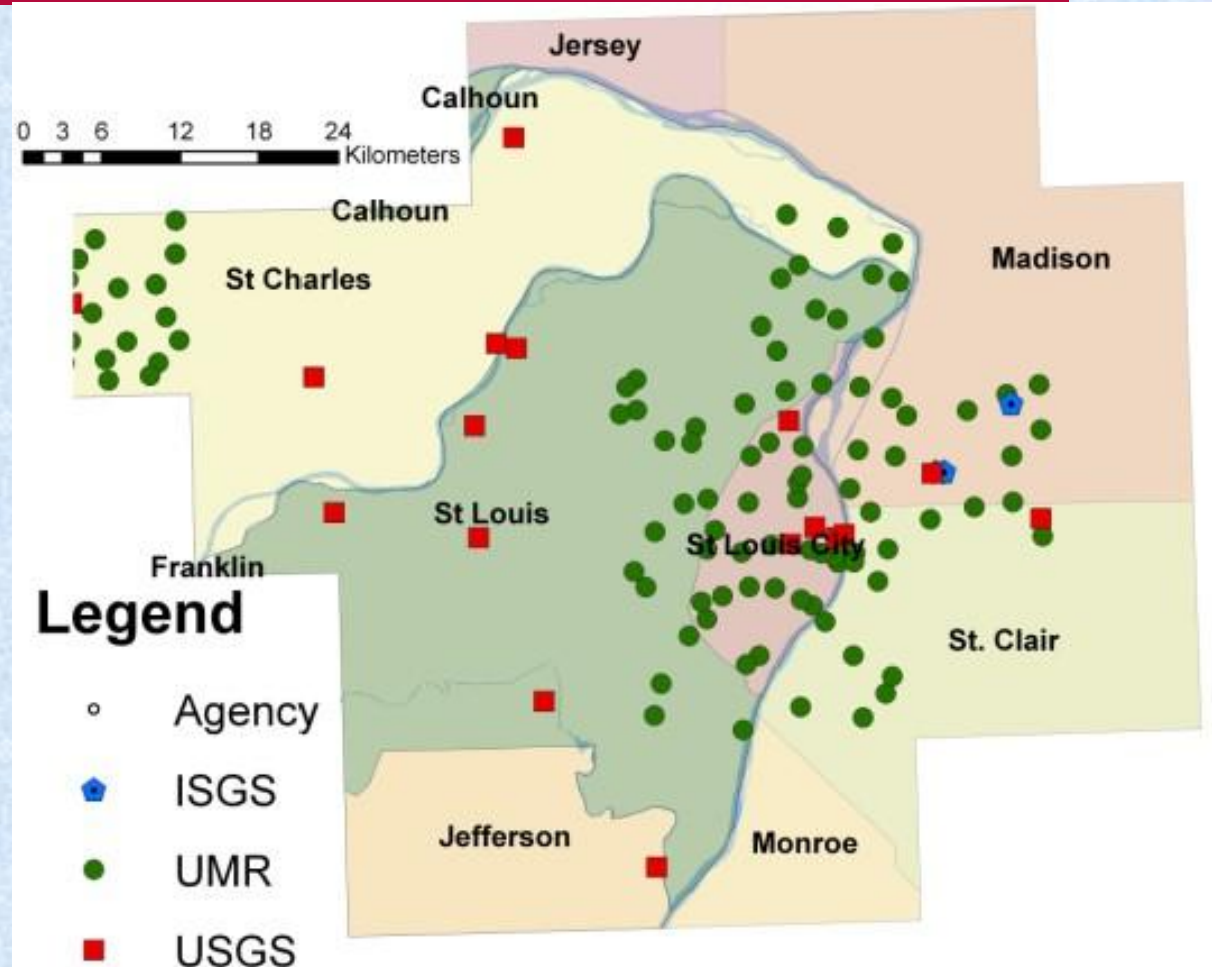
- Data Sources (Digital Format); MoDNR-DGLS and ISGS

State	Borehole type	Number of records	Item
Missouri	Bedrock	2338	Depth to bedrock, Bedrock type
	Corelog	729	Core recovery (%), Rock Quality Designation (RQD)
	Grain Size	93	Grain size analysis of soil
	Material	2330	Description of soil material
	Physical Property	1906	Standard Penetration Test (SPT) N-value, Cone Penetration Test (CPT), ASTM class, Unit weight (water content,%), Liquid limits, and Plastic index
	Water Observation	961	Depth to groundwater
	Site	2394	
Illinois	Highway Log	857	Description of soil material
	Highway Engineering	496	Standard Penetration Test (SPT) N-value
	Highway Head	2226	Description of geotechnical boring
	Log	3636	Description of soil material
	Water Well	4728	Description of water well
	Site	4817	

Locations of Shear Wave Velocity (V_s) Measurements

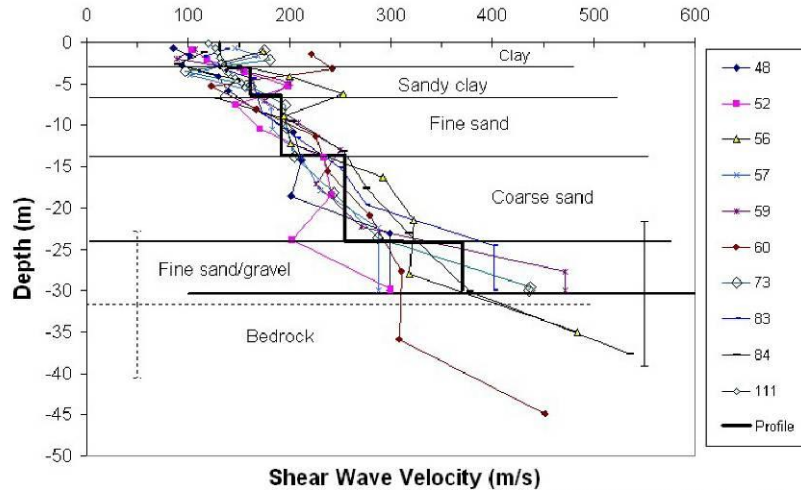
Data Sources (119):

- **ISGS (3)**
- **UMR (99)**
- **USGS (17)**
- **More being added to the database from private sector consultants**



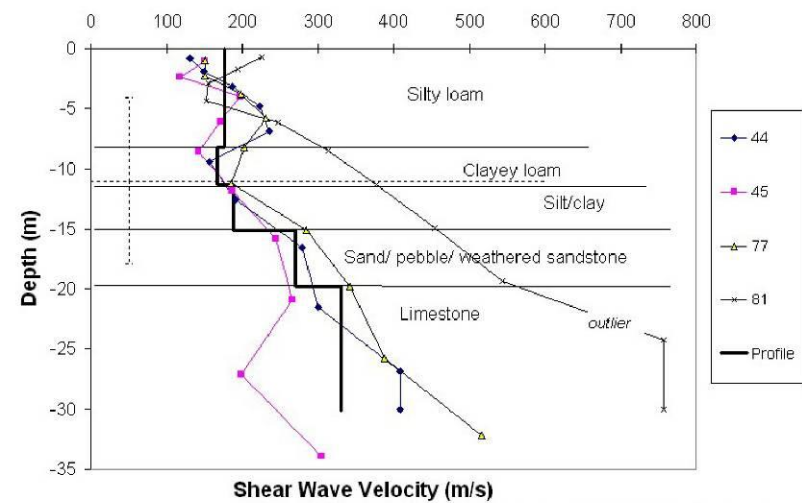
Vs Reference Profiles and Soil Columns derived from adjacent boreholes

**Vs Profile-Cahokia Clayey
(Monk Mound, Granite City, Cahokia Quads)**



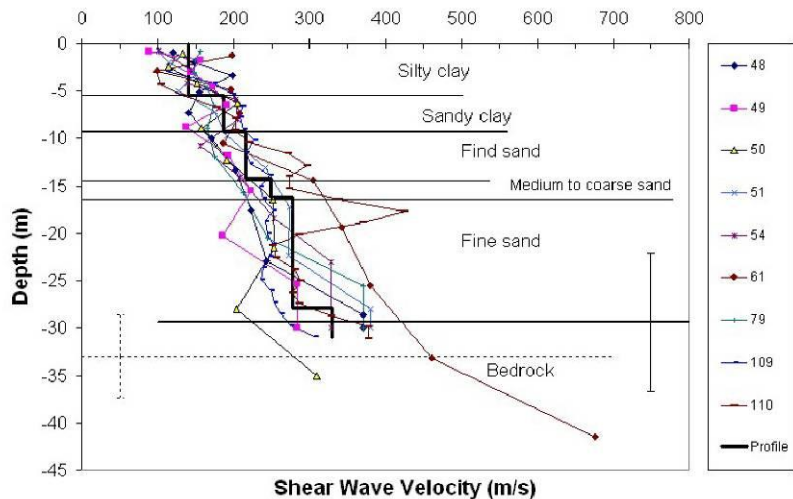
boring Log(121190266700), 109m from #73

Vs Profile-Loess in Illinois



boring log(121632937000), 40m from #77

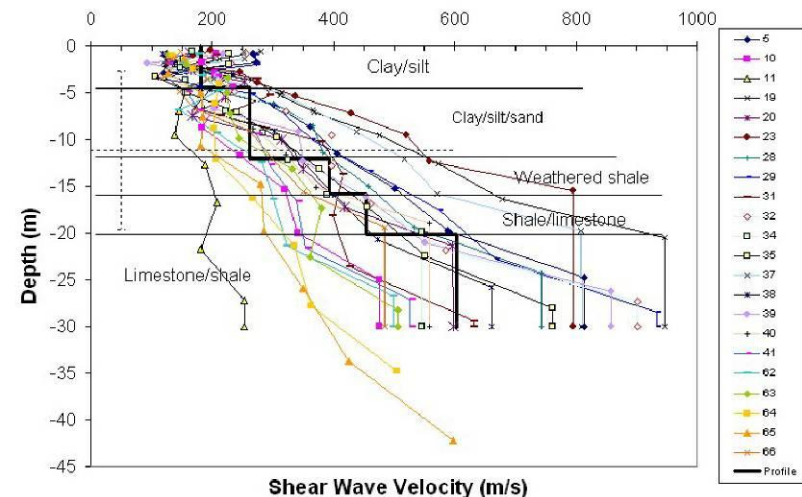
**Vs Profile-Cahokia Sandy
(Monk Mound & Granite City Quads)**



boring Hwylog(121192642900), 54m from #48

Examples

Vs Profile-Loess in St. Louis



boring IS70_A3745U17+732R, 400m from #38

Geospatial Prediction of the Groundwater Table

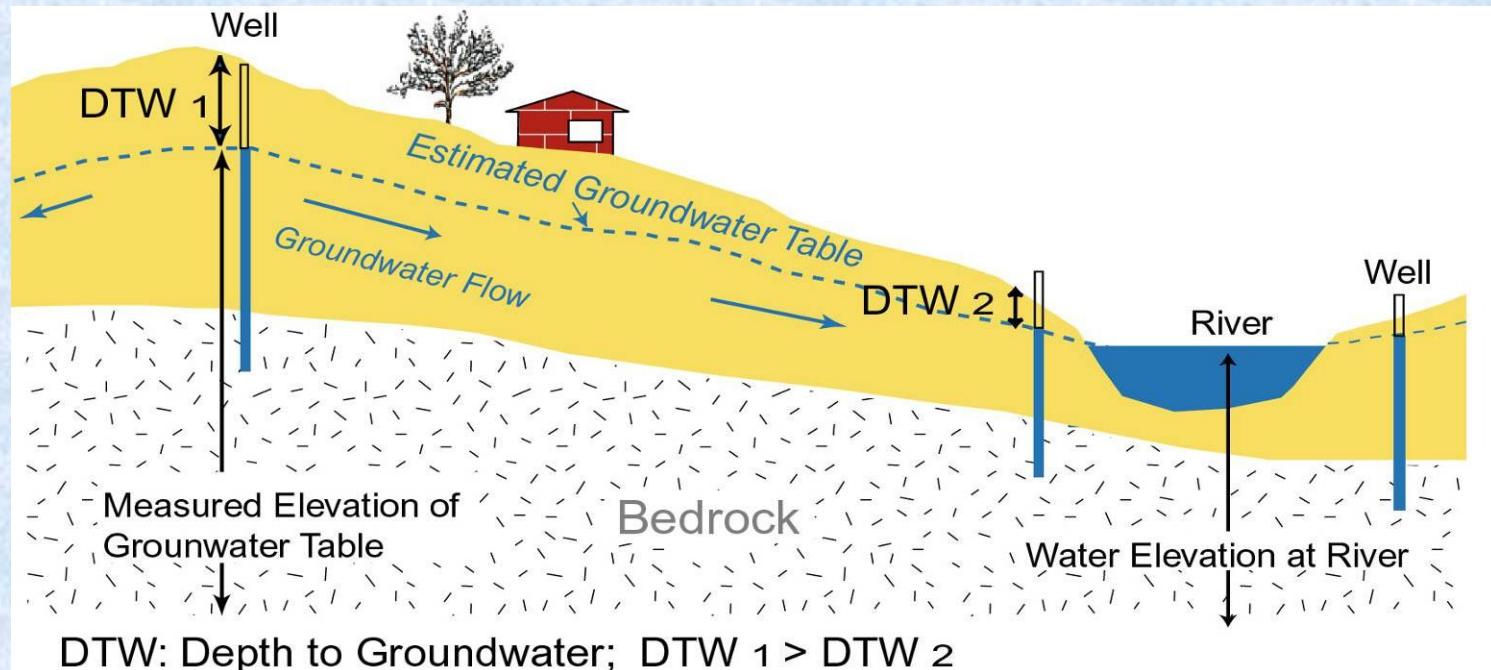
Application: important consideration in engineering and environmental decision making; for

- **waste disposal sites**
- **natural hazards, such as shaking-induced soil liquefaction, and lateral spreads.**

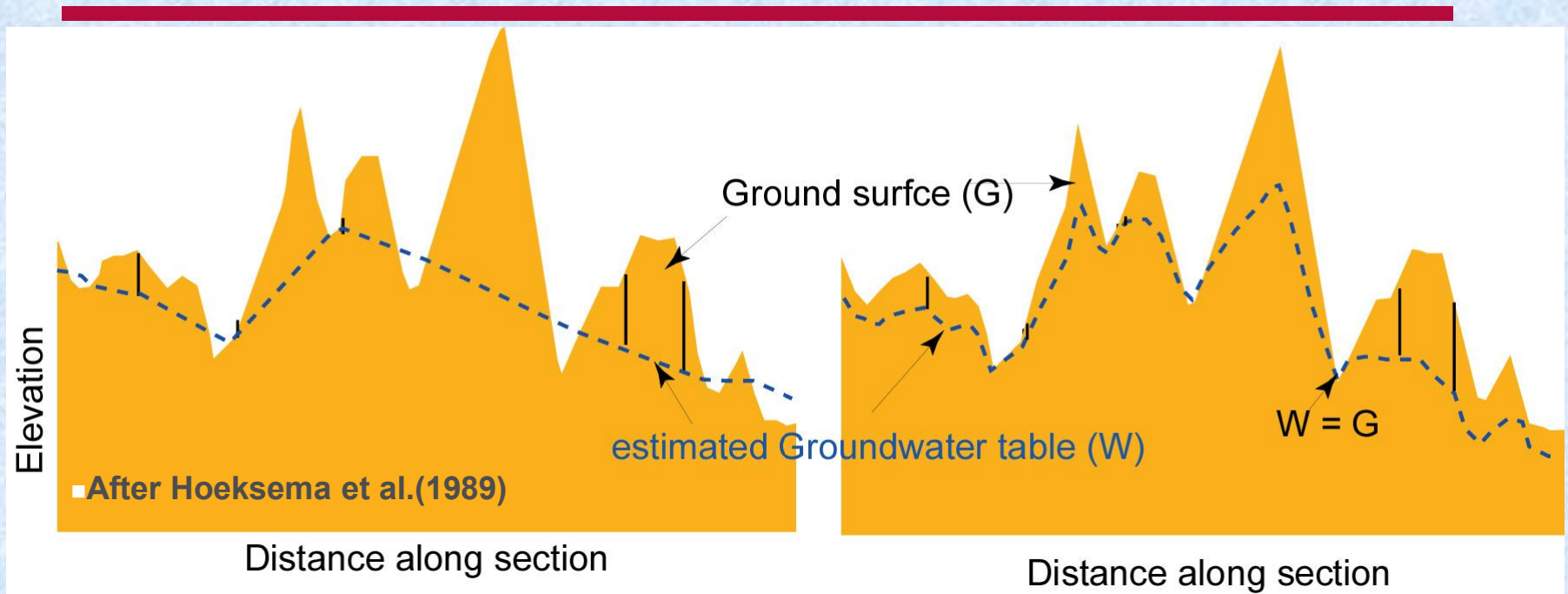
General Specifications of the Groundwater Table

The **groundwater table elevation** generally meets the following specifications:

- 1) follows the shape of the land surface
- 2) is equal to the **ground elevation** at streams,
- 3) the depth to **groundwater table** is deepest in hilly area



Profile of Groundwater Table (W) with and without considering the ground surface (G)



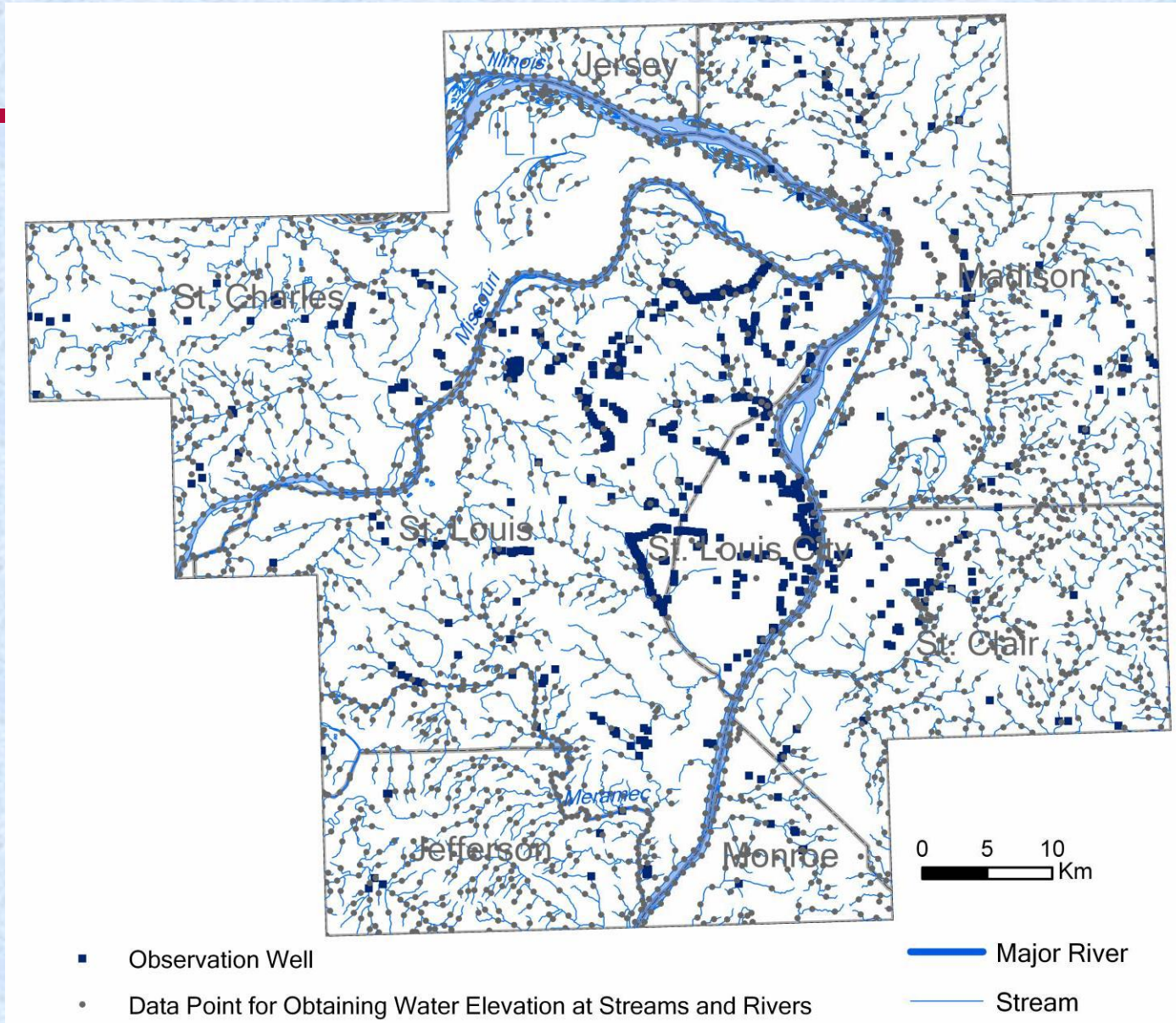
Estimated W without considering G

- Using kriging

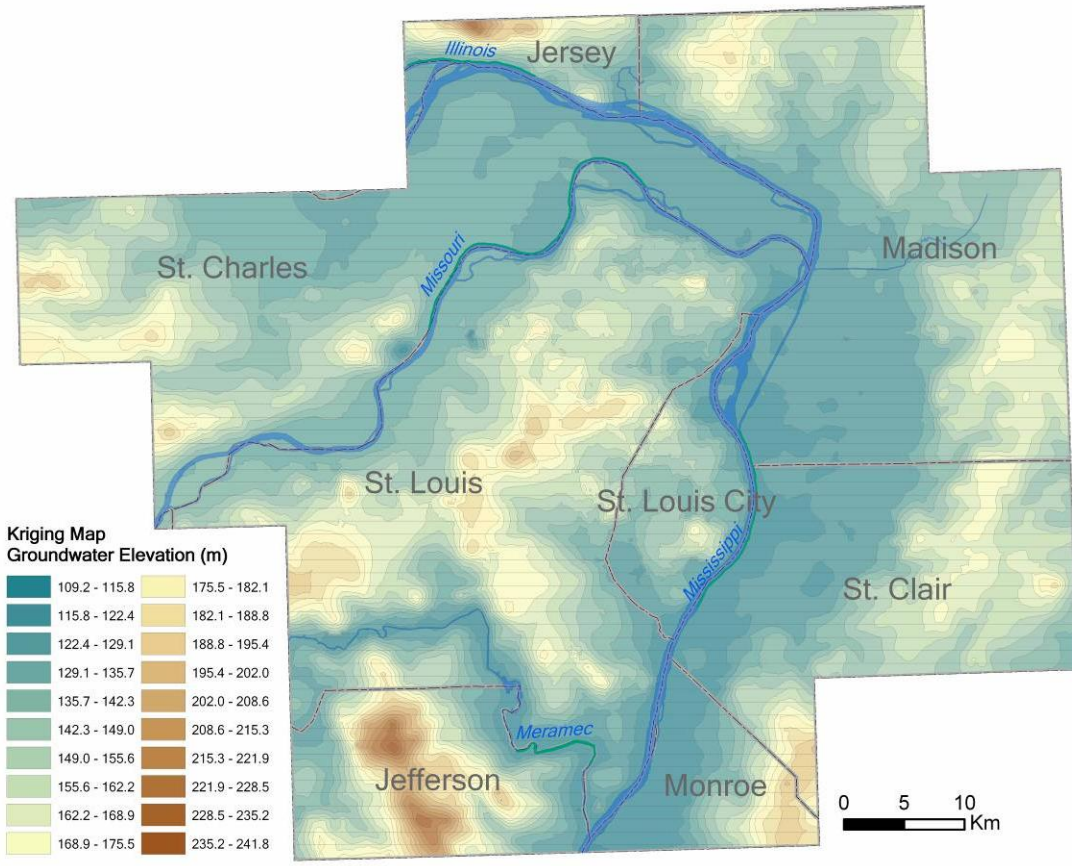
- Estimate W concerning G and constraining $W=G$

- Using cokriging

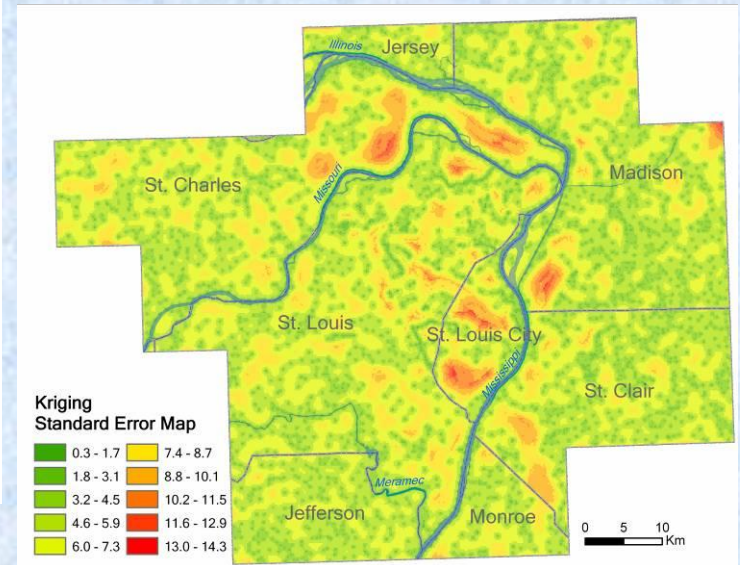
Input data for Modeling Groundwater Table



Kriging Map of Predicted Groundwater Table Elevation



Standard Error Map Using Kriging



■ Raster data model

Cokriging Map of Groundwater Table

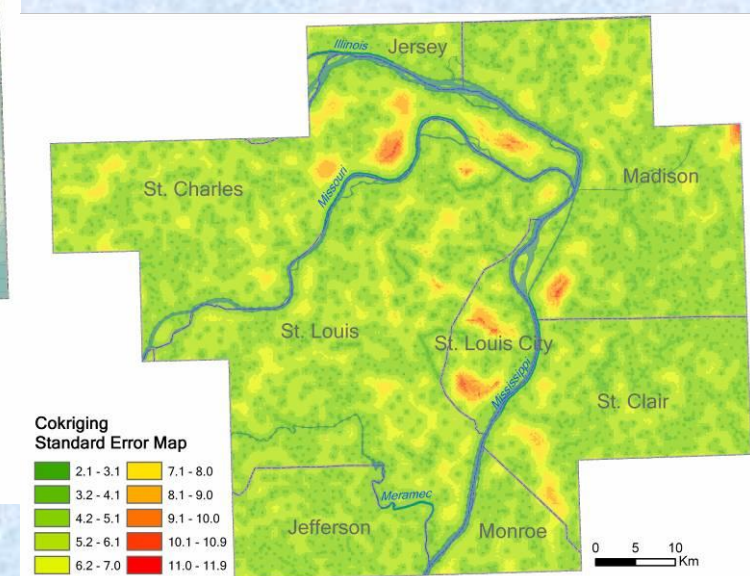
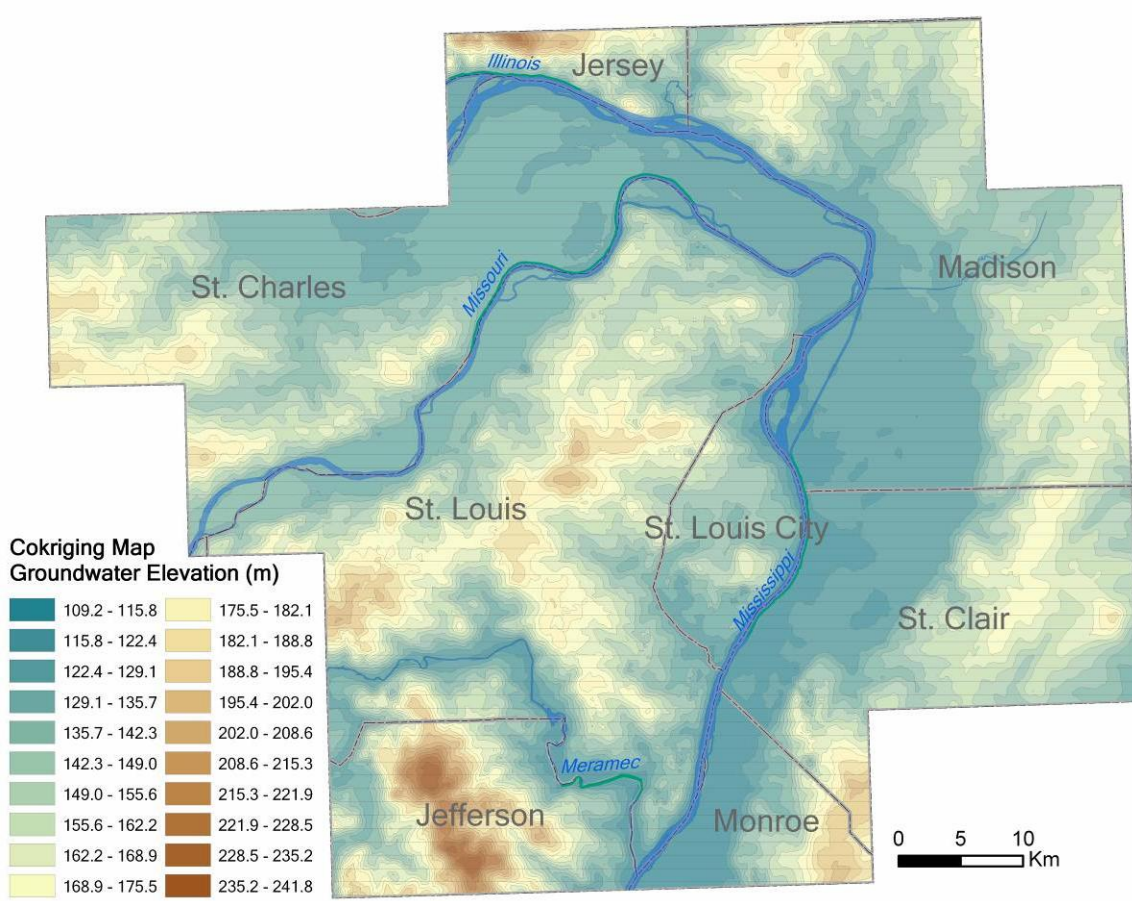
Primary variables:

- **1,052 well logs**
- **2,569 artificial data points along drainage.**

Secondary variables:

- **Resolution/accuracy of actual ground elevations (500m × 500m grids), extracted from USGS Digital Elevation Models**

2) Cokriging Map of Predicted Groundwater Table



■ Raster data model

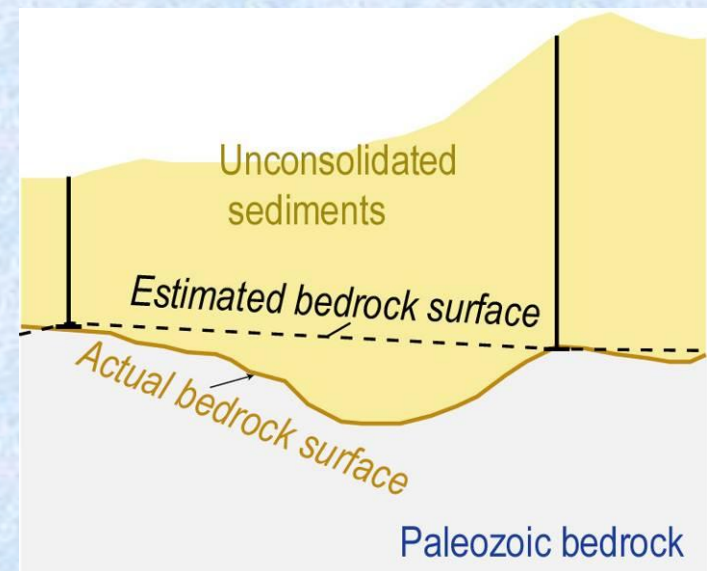
Standard Error Map Using Cokriging

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Problems with interpolating the bedrock Surface beneath the ground

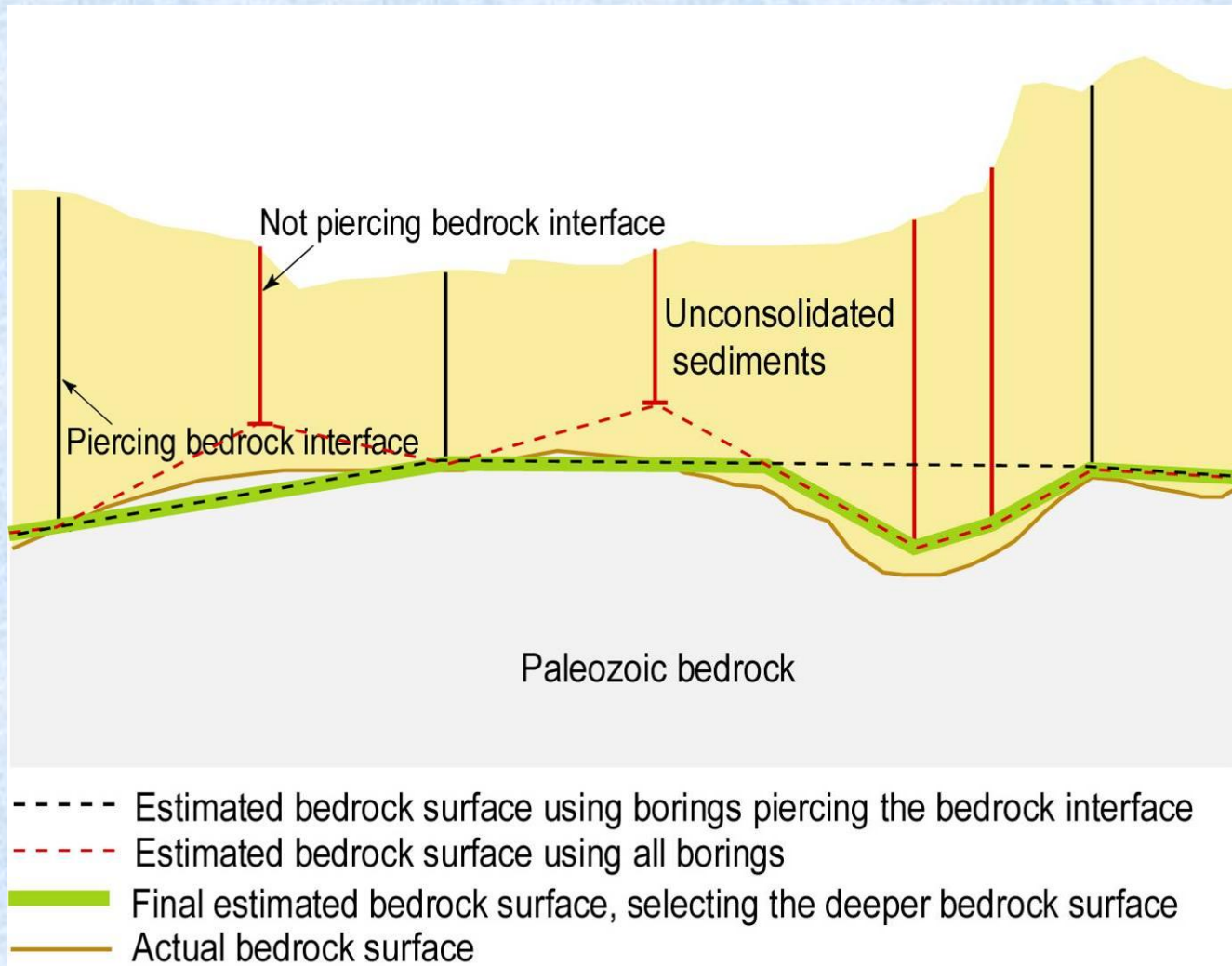
In undulating terrain, the **bedrock surface** is often a complex, undulating feature, shaped by previous erosional and deformational events

- Linear interpolations between adjacent data points in rugged terrain often lead to erroneous results, because:
 - 1) overestimation of bedrock surfaces in paleovalley systems
 - 2) a local contouring model may result in poor estimates when applied to a different geomorphic province or terrain



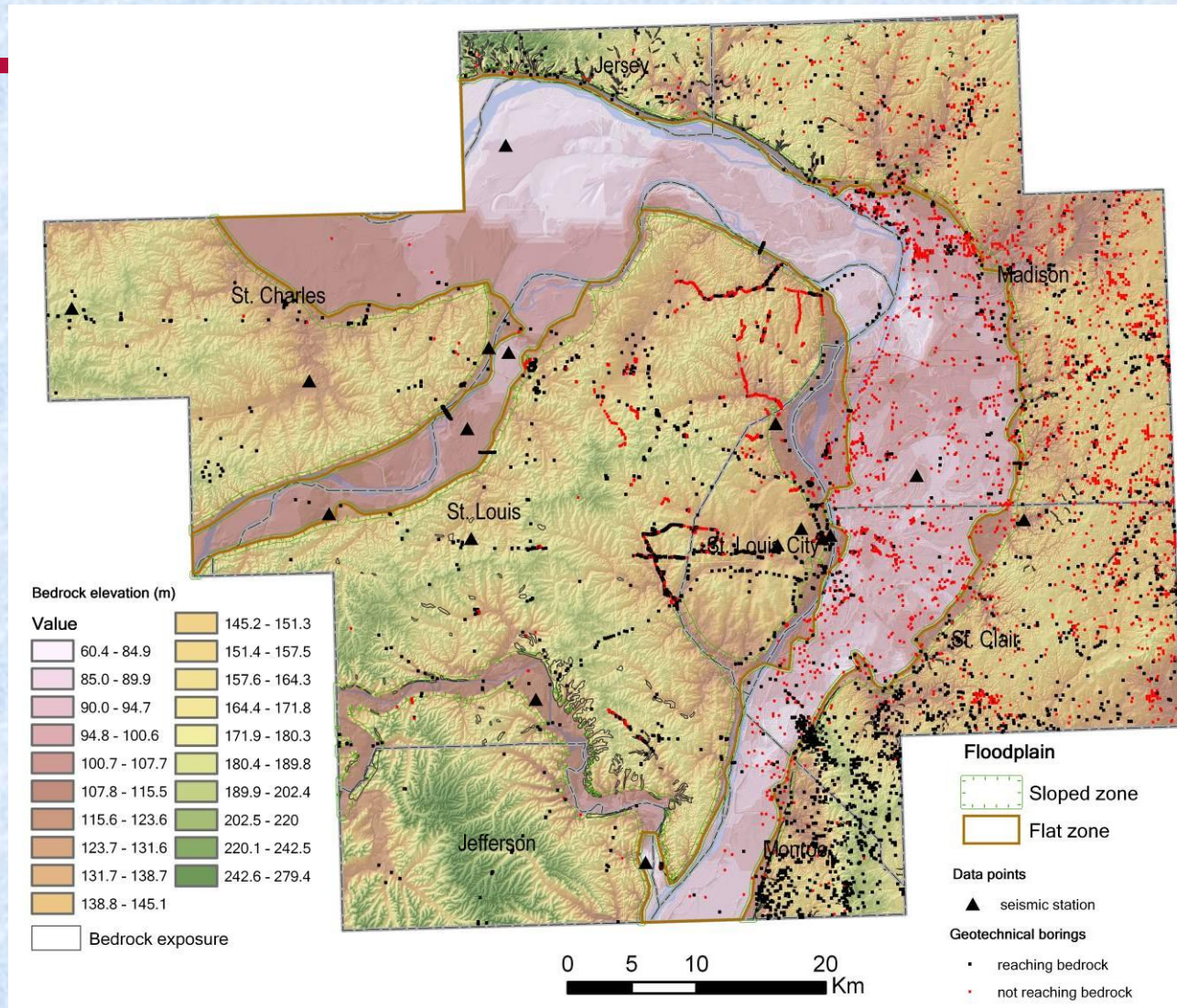
Procedure for Interpolating Depth-to-Bedrock

3) Of these two approximations, our model was programmed to select the *deeper bedrock surface*, which we feel is more accurate



Kriging Map of Bedrock Elevation

subtracted DEM from kriged Depth-to-Bedrock



Bedrock elevation (m)

■ Raster data model

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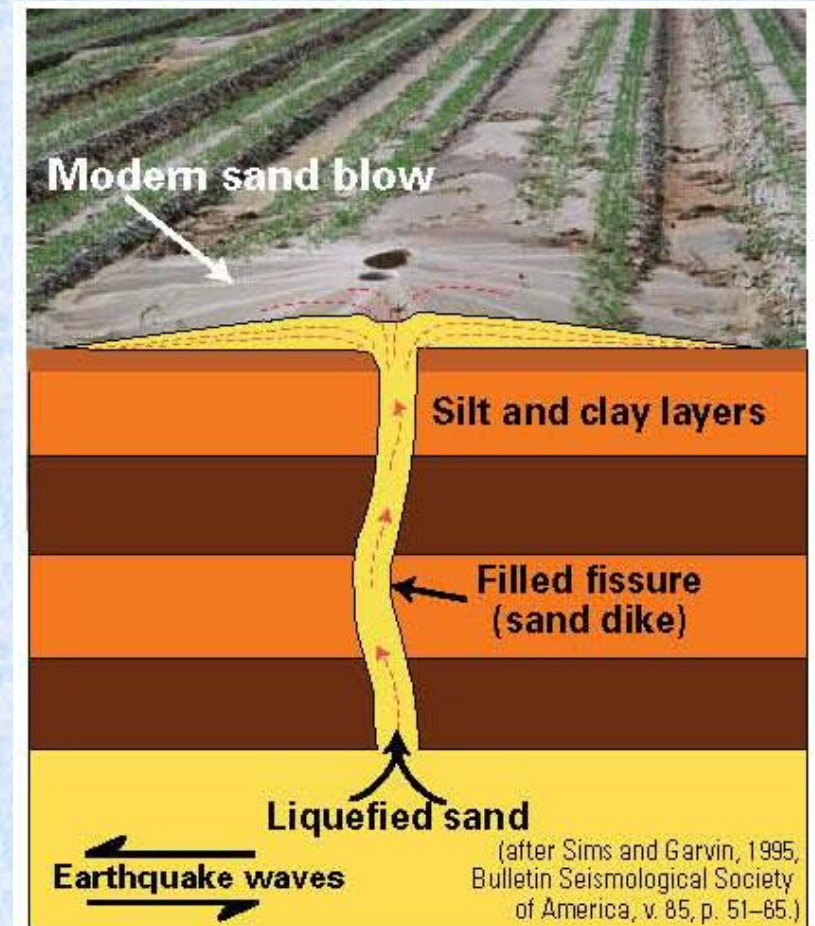
Step 2

Preliminary Assessment of Soil Liquefaction Potential

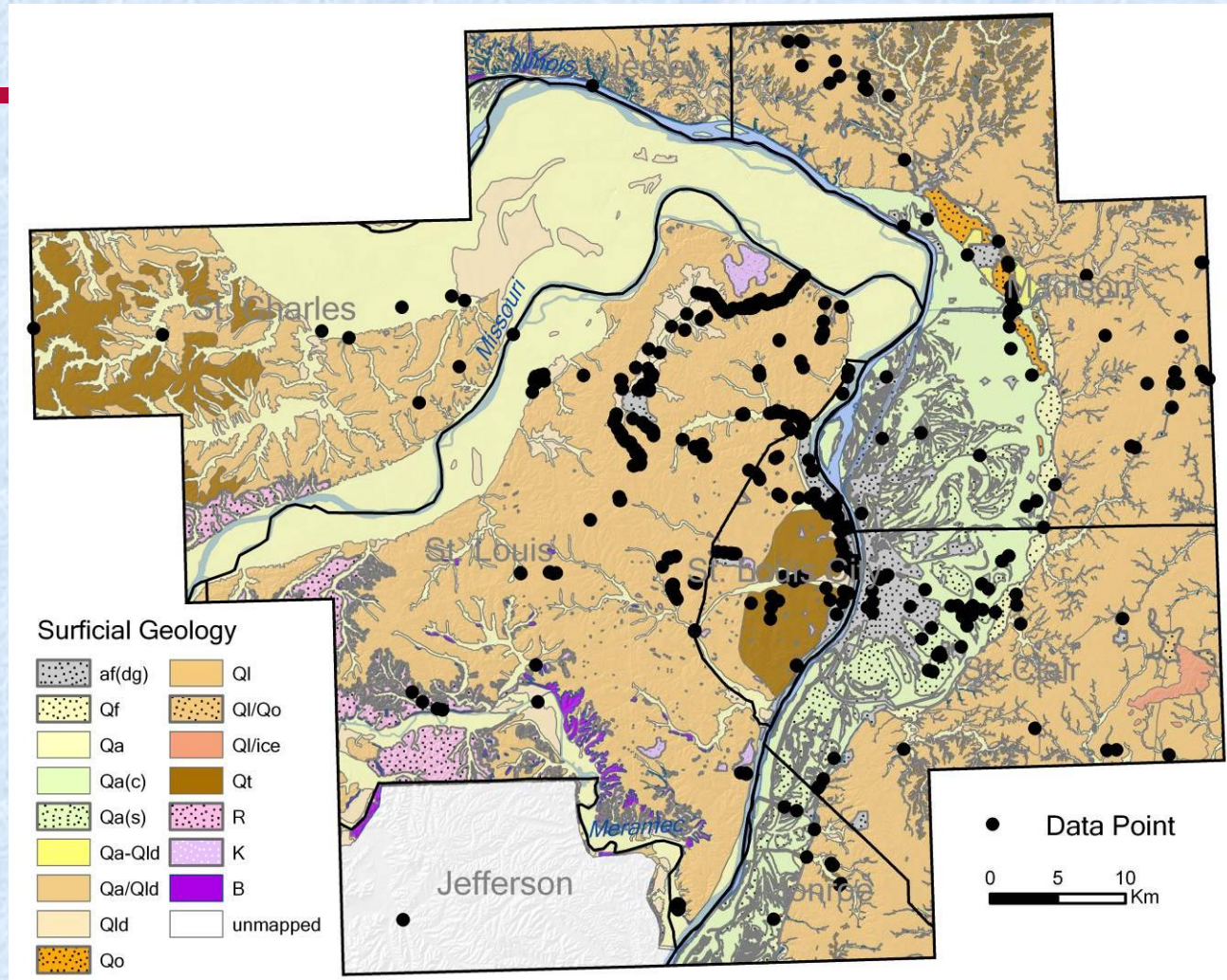
Liquefaction is a **soil failure mechanism** that occurs when saturated cohesionless soil loses shear strength. This occurs when the soil pore pressure exceeds the effective confining stress.

It often occurs in loose unconsolidated sands during earthquake-induced ground shaking, and **behaves like a fluid**.

When the water pressure increases and sand is liquefied, a **slurry of sand/water** is forced to the ground surface.



Locations of 564 Borings used to calculate the Liquefaction Potential Index, or LPI



- **Data Sources (Boring information):**
 - MoDNR-DGLS, ISGS

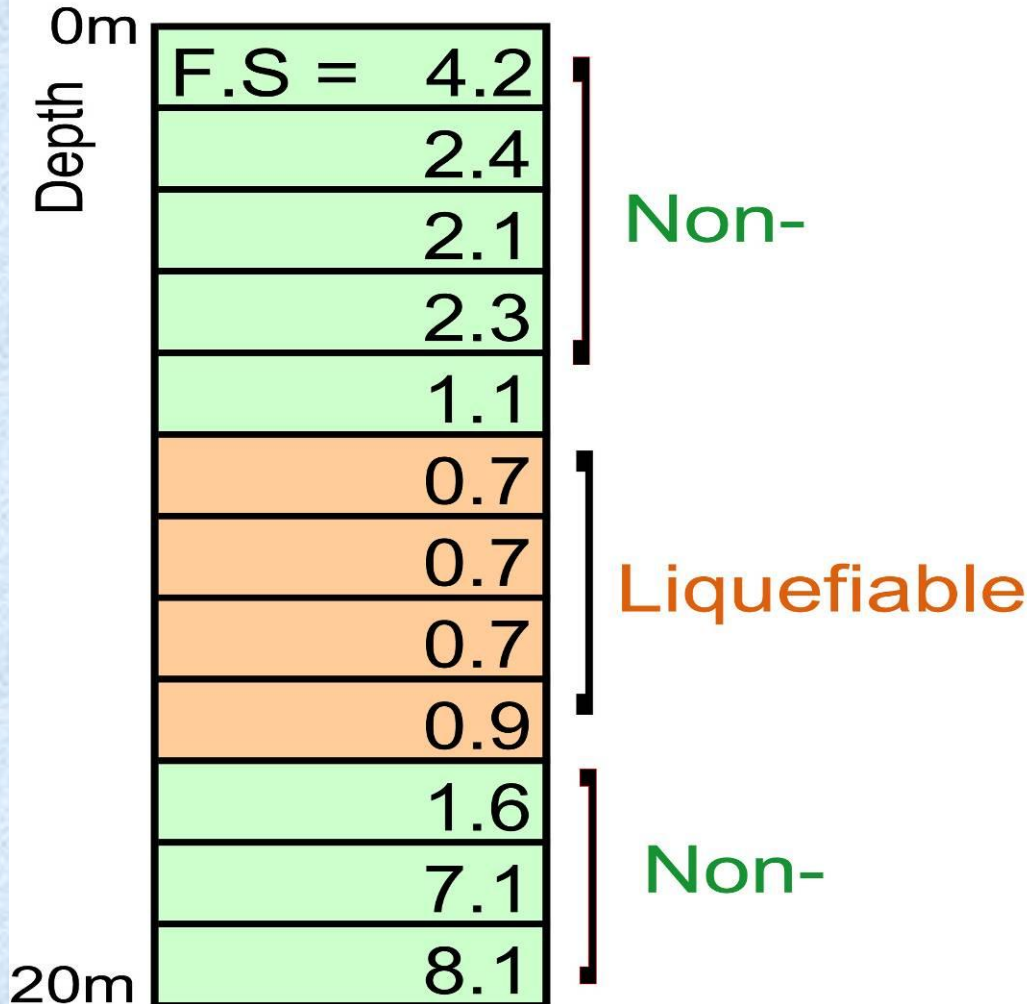
Historical Liquefaction Severity Assessed from LPI (Iwasaki, 1982)

LPI	Severity of Liquefaction
0	None
$0 < \text{LPI} \leq 5$	Little to none
$5 < \text{LPI} \leq 15$	Moderate
$15 < \text{LPI} \leq 100$	Severe

- The LPI technique evaluates the entire soil column overlying the stable bedrock
- The higher the LPI value, the more severe liquefaction damage.

Advantage of LPI method over FS

Factor of Safety

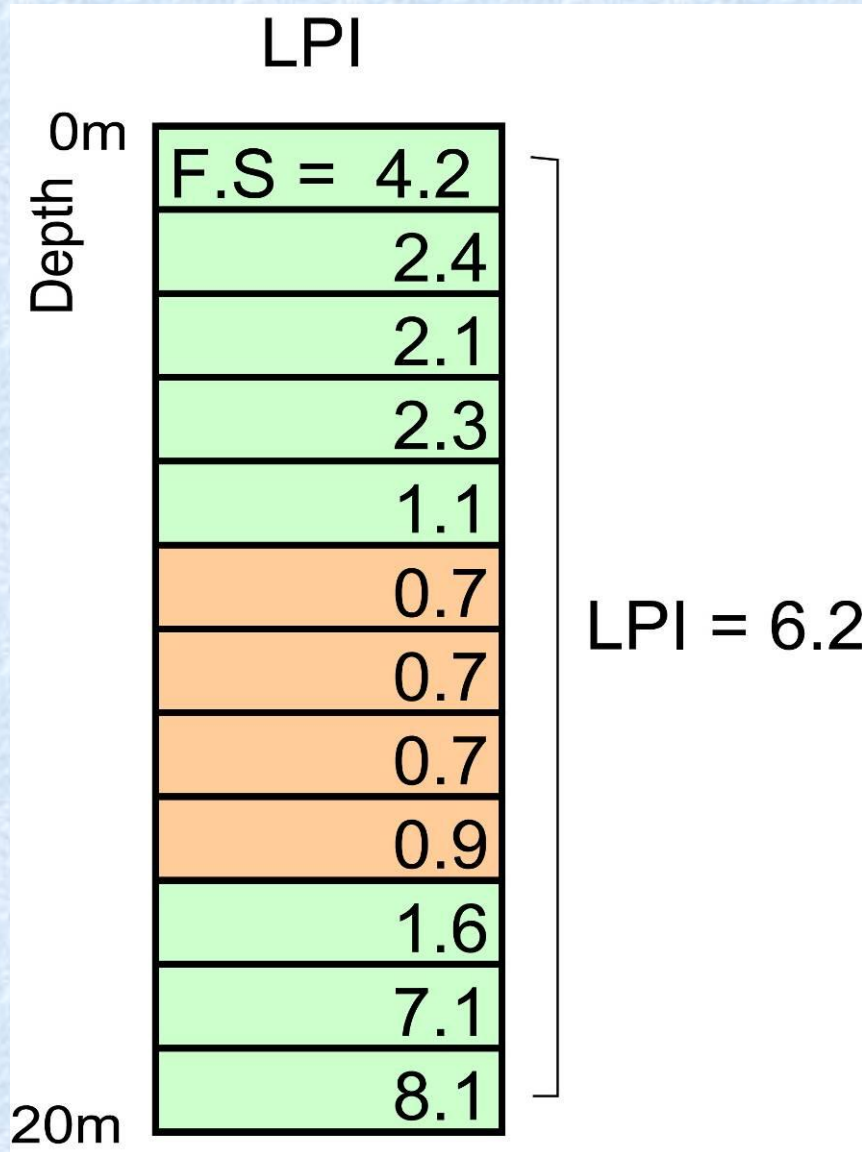


Where the mixture of a **liquefiable** and non-liquefiable soil layer exists at a single boring,

Will liquefaction occur?

If so, how severe is the liquefaction?

The LPI Method allows us to subjectively grade the severity of liquefaction potential



LPI = 6.2 in this soil column;

therefore, liquefaction is likely to occur

Liquefaction severity will be “MODERATE**”, based on historical liquefaction evidences (Iwasaki et al., 1982)**

LPI estimates for various Earthquake Scenarios

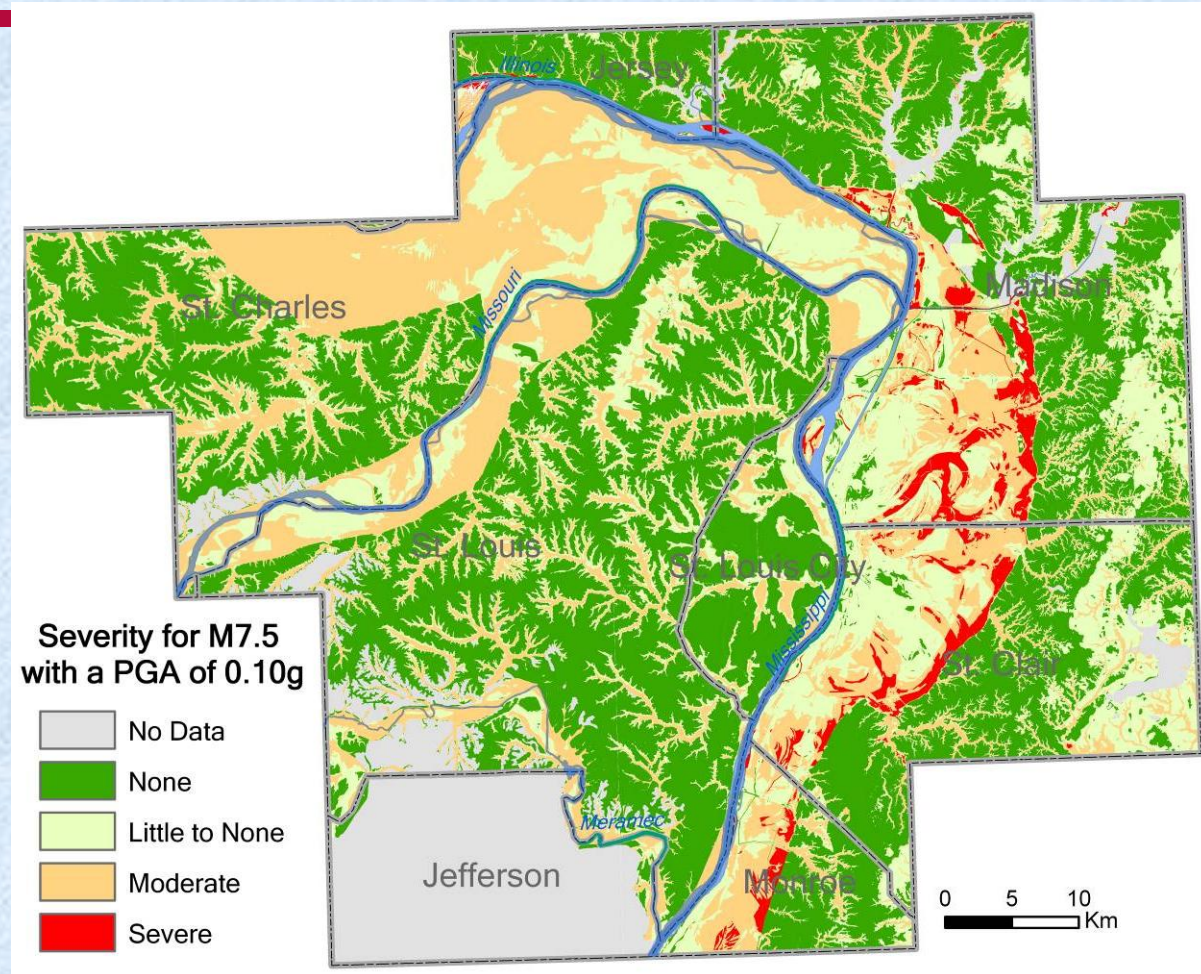
Liquefaction potential in the upper Mississippi Embayment may not be a significant issue at Magnitudes < 6.4 (Obermeier, 1989; Tuttle and Schweig, 1995)

LPI values from 564 data points were calculated for a M7.5 quake with PGA values of 0.10g to 0.30g (Toro and Silva, 2001), emanating from the *New Madrid Seismic Zone*

Liquefaction Potential Map (inferred from LPI) for M7.5 with 0.10 PGA

Severe Liquefaction Potential Area (LPI > 15):

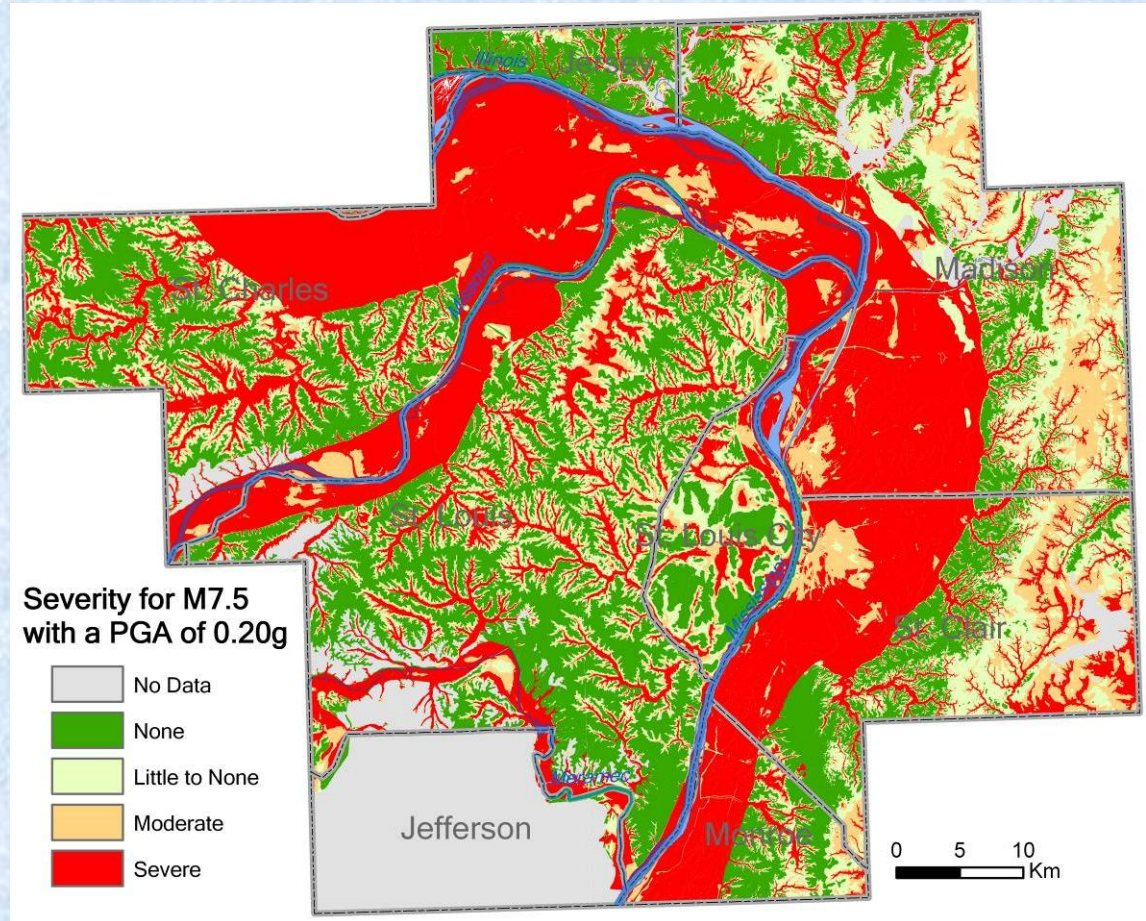
- Alluvial fans in part (where, **gwt < 0.5m**) in Illinois
- Near confluence of Mississippi-Illinois rivers



- Grey areas have insufficient number of borings to analyze

Liquefaction Potential Map (inferred from LPI) for M7.5 with 0.20 PGA

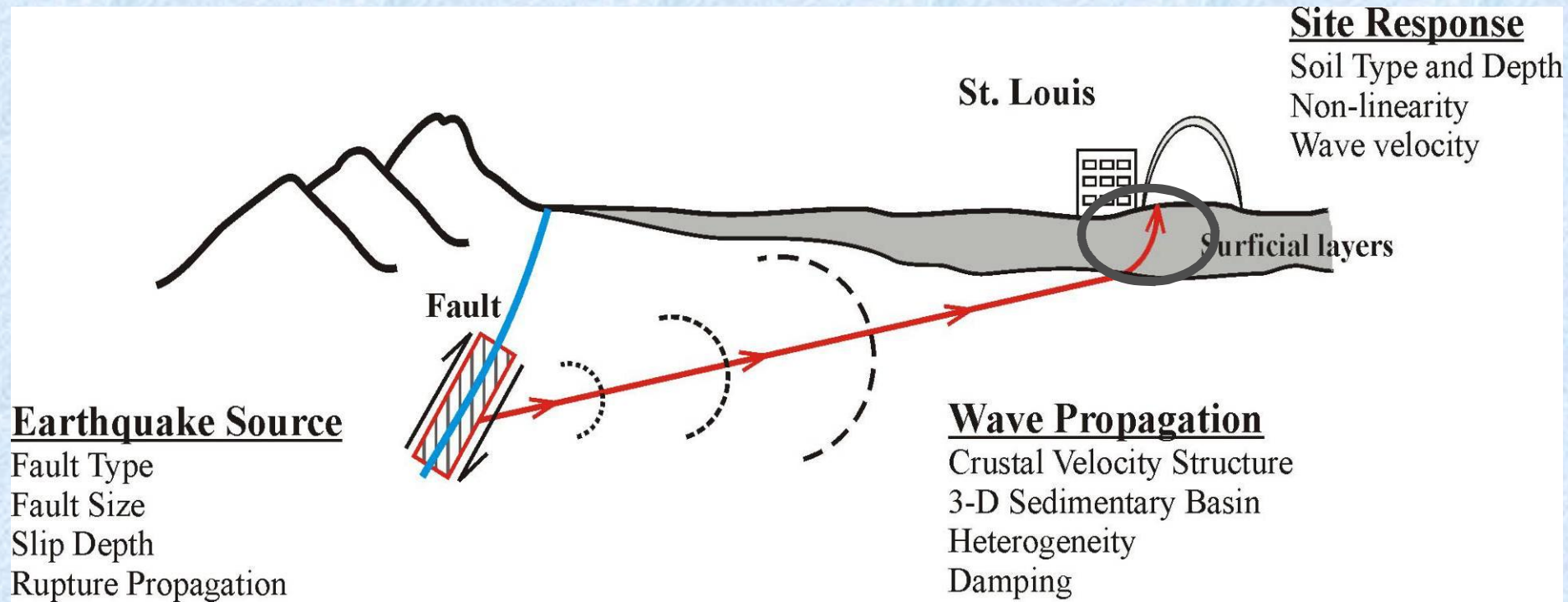
- Severe Liquefaction Potential Area (LPI>15):
 - Alluvial fan in part (gwt<4.7m) in Illinois
 - Alluvium in part (gwt<4.4m) along major rivers and streams
 - Clayey alluvium (gwt<4.6m) and sandy alluvium (gwt<5.1m) in ox bow & adjacent alluvial fan



Step 3

Physical Factors Affecting Seismic Site Response

What is Site Response? How the soil under the site affects the intensity of ground shaking.



The type, depth and size of fault, combined with physical properties of crust and geophysical properties of the surficial soils affect **site response**.

Ground Motion Parameters

Peak Ground Acceleration (PGA) is the maximum acceleration experienced by the particle during the course of the earthquake motion.

Spectral Acceleration (SA) what is experienced by a building, as modeled on a massless vertical rod, having the same natural period of vibration as the building.

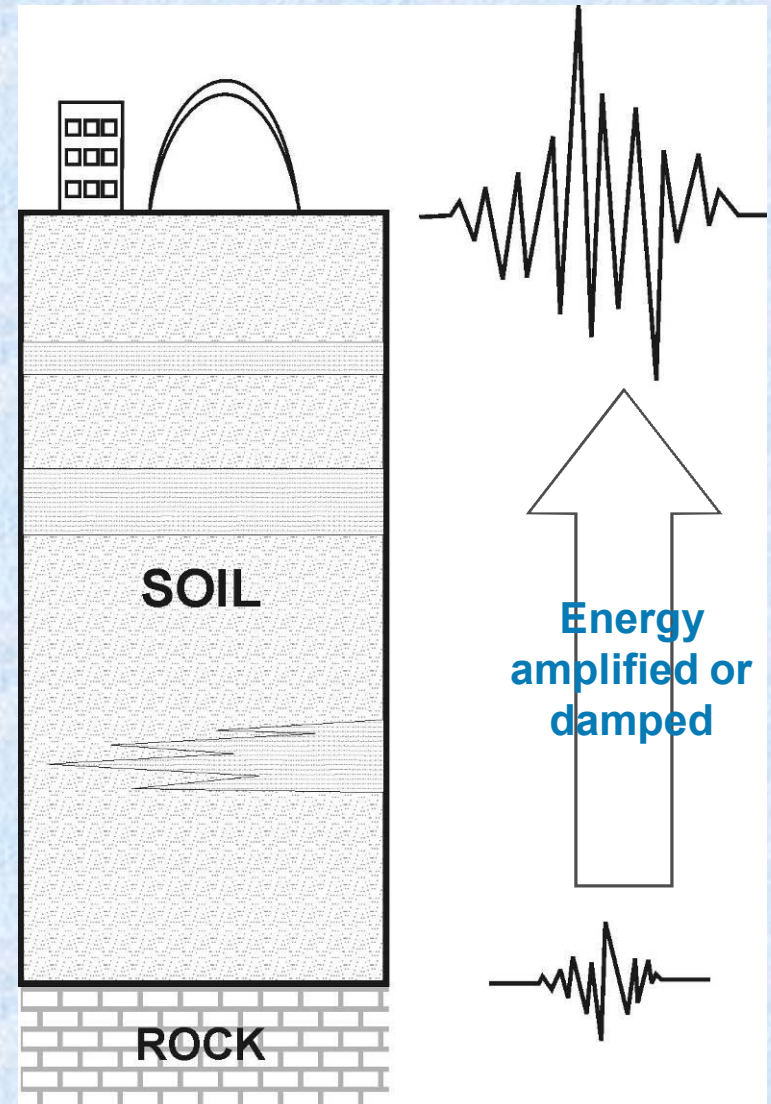
Estimating surface accelerations

Surface accelerations can be estimated using 1-D seismic site response software

Typical input data includes:

- **Soil physical properties**
- **Soil dynamic properties**
- **Soil thickness**
- **Input rock motion at the base of the soil column**

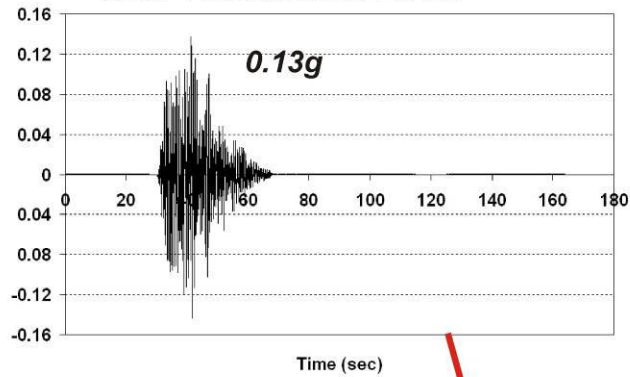
These are combined to estimate the site amplification, or de-amplification



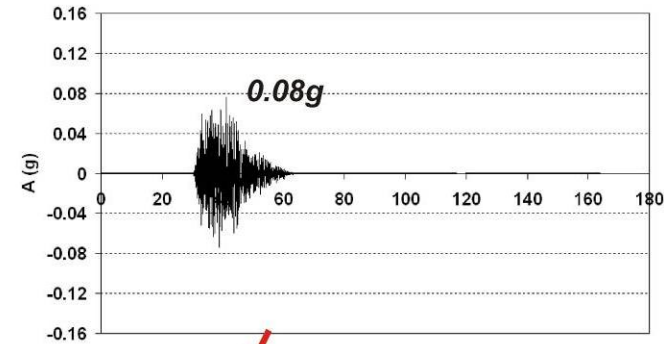
Effect of Soil Thickness on Peak Ground Acceleration (PGA)

Magnitude 6.8 quake
emanating from
South Central Illinois
at 110 km

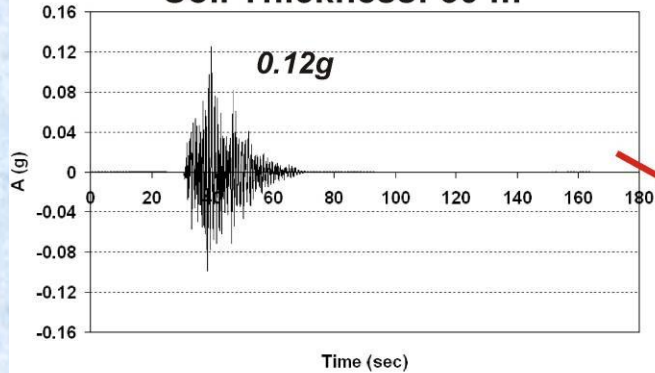
Soil Thickness: 28 m



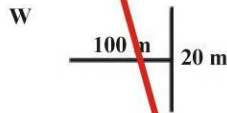
Soil Thickness: 22 m



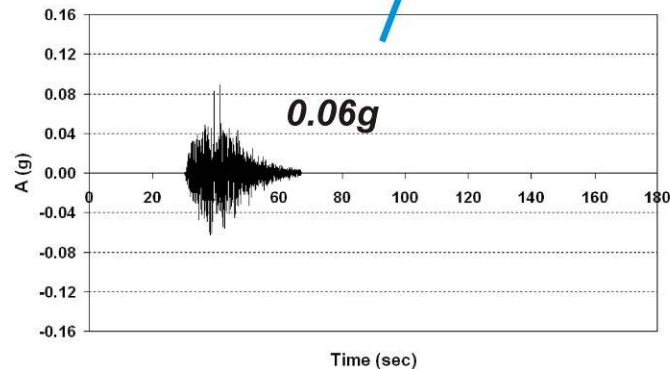
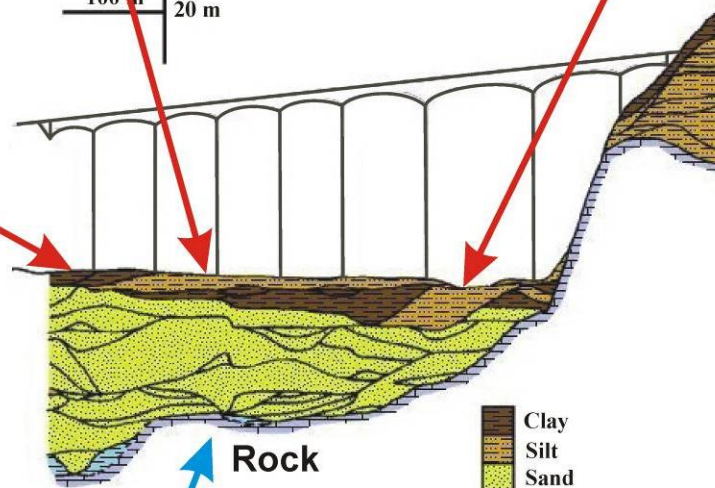
Soil Thickness: 39 m



W

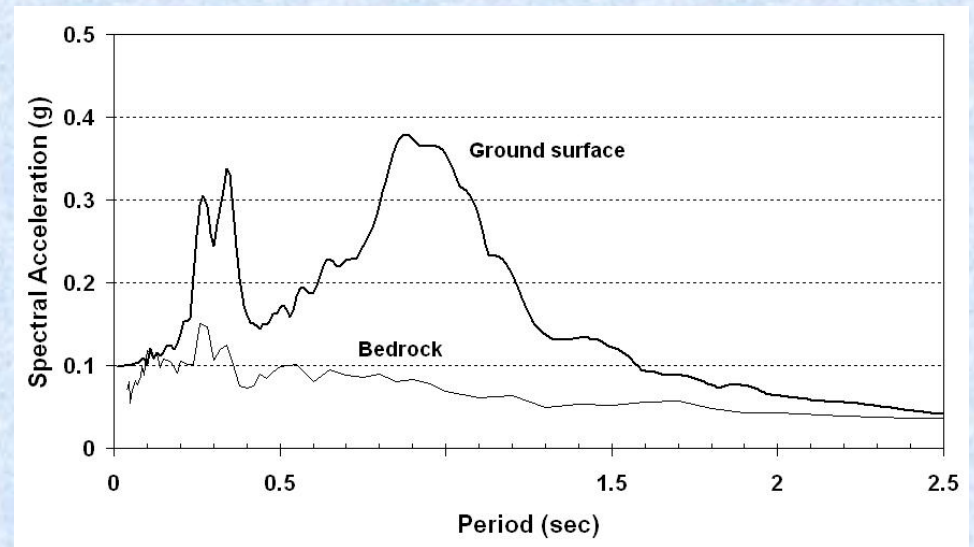
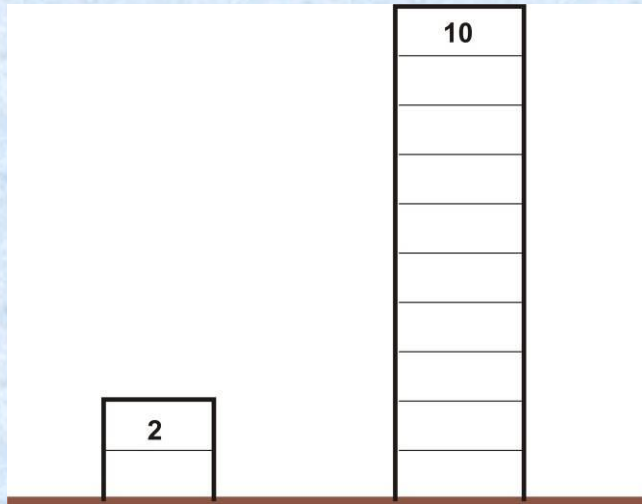


E



Spectral Accelerations (SA)

The spectral acceleration value varies with the natural period of the structure.

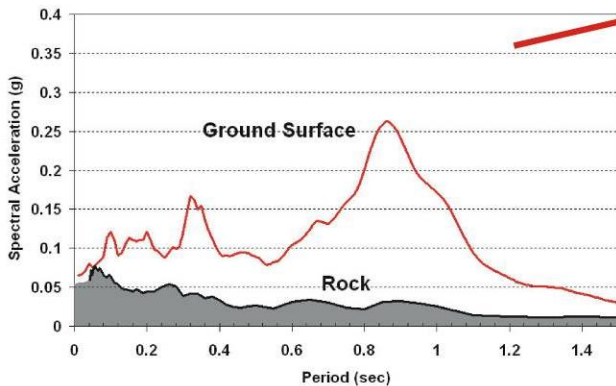
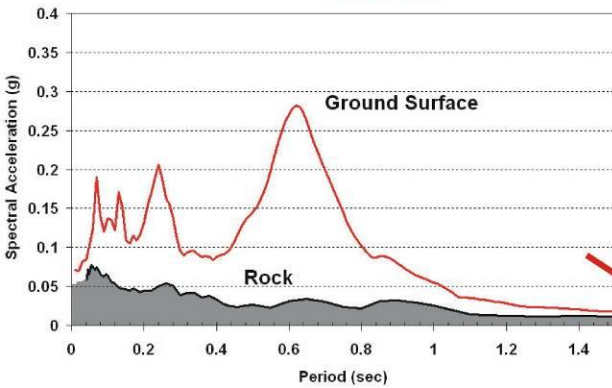


(approximately related)

Effect of Soil Thickness on RESPONSE SPECTRA

Soil Thickness: 28 m

Peak SA = 0.28 g
Peak Period = 0.62 sec

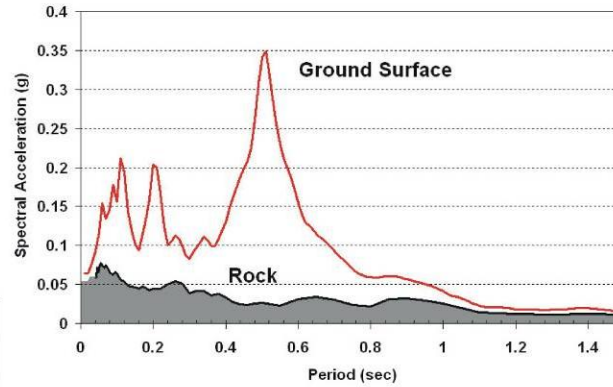


Soil Thickness: 39 m

Peak SA = 0.26 g
Peak Period = 0.87 sec

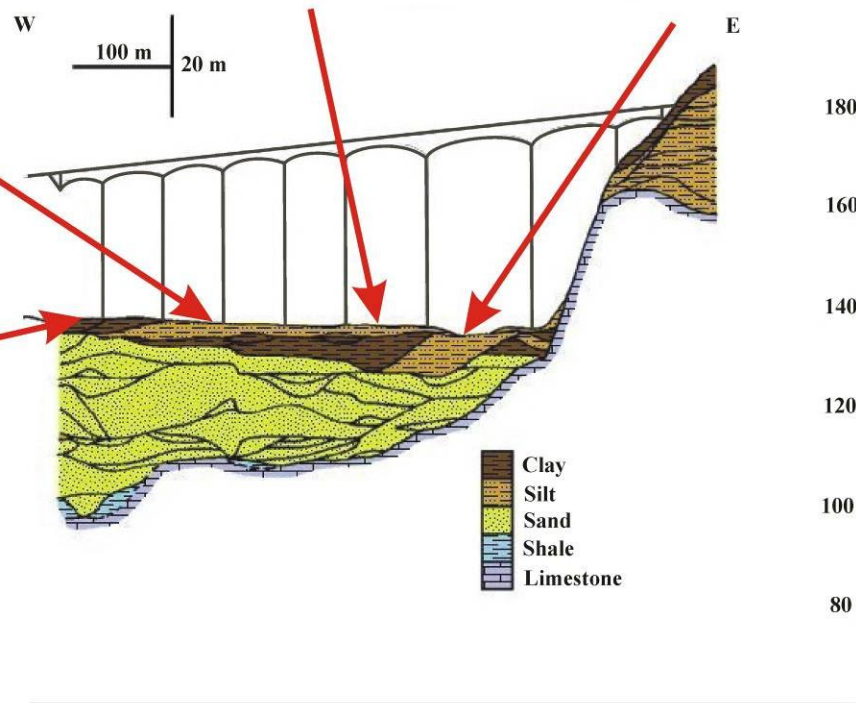
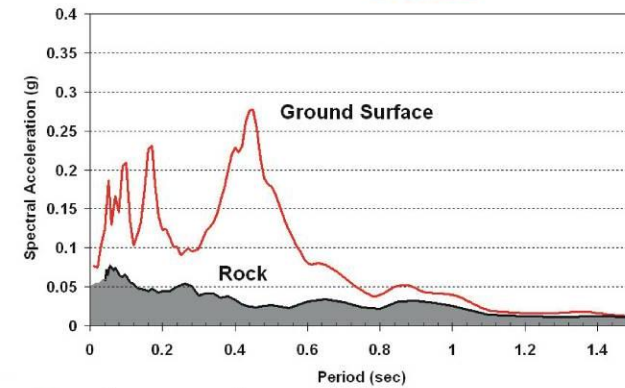
Soil Thickness: 25 m

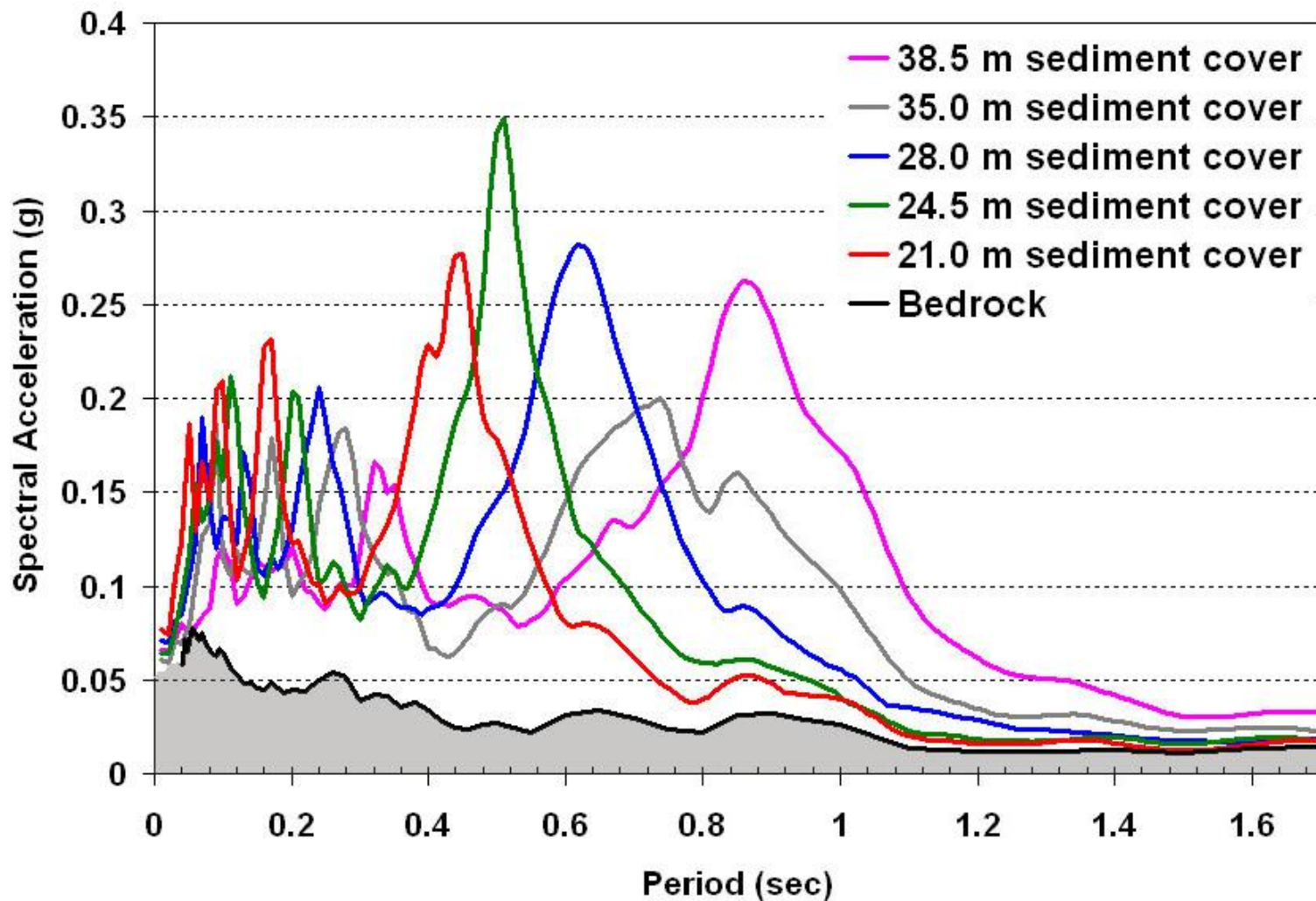
Peak SA = 0.35 g
Peak Period = 0.51 sec



Soil Thickness: 22 m

Peak SA = 0.28 g
Peak Period = 0.45 sec





Variation in expected *spectral acceleration* with *alluvial thickness* in the St Louis, MO area

Step 4

Distribution of Site Amplification and Development of Site Amplification Maps

The Missouri S&T pilot study sought to develop the following maps, of a ~ 460 km² land area:

- 1) **Site amplification maps** for different levels of ground shaking (0.01, 0.05, 0.1, 0.2, 0.3, 0.4, 0.5, 0.6, 0.8, 1.0) in terms of PGA, 0.2 sec and 1 sec spectral accelerations.
- 2) **2%** probability of exceedance in 50 years in terms of PGA;
- 3) **5%** probability of exceedance in 50 years in terms of PGA;
- 4) **10%** probability of exceedance in 50 years in terms of PGA;
- 5) **0.2 second** spectral accelerations for **2%, 5% and 10%** probabilities of exceedance in 50 years;
- 6) **1 second** spectral accelerations for **2%, 5% and 10%** probabilities of exceedance in 50 years;
- 7) 2 scenario earthquakes (M_0 7.0 and 7.7) and their associated **PGA** and **0.2 sec-SA** and **1 sec-SA**;

What information do we need to estimate site amplification?

1) Characterize the shallow geology overlying the bedrock

Surficial geology maps

Depth to Bedrock

2) Characterize the bedrock acceleration

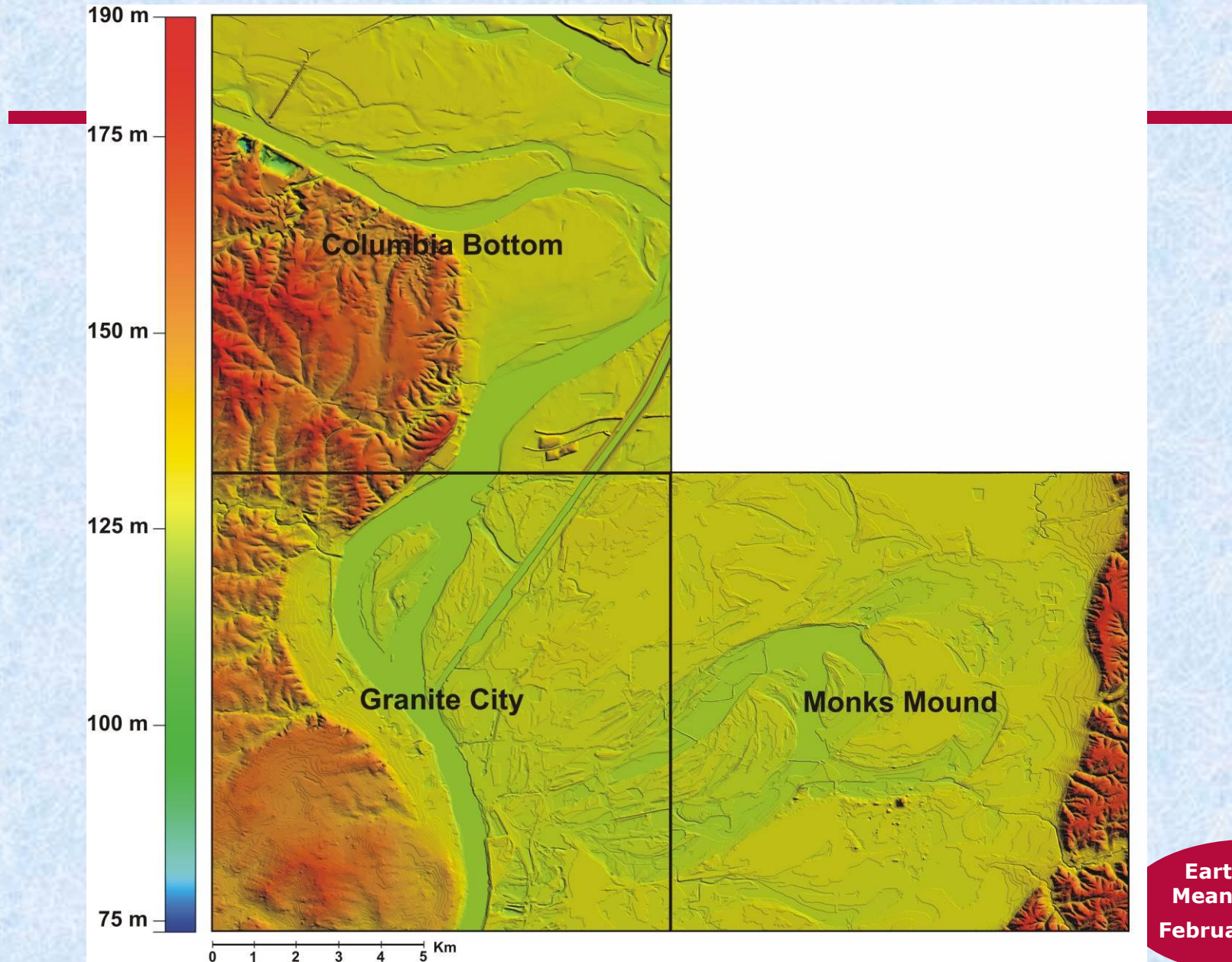
3) Characterize the thickness and shear wave velocity of the bedrock underlying the surficial materials

4) Characterize the properties of the surficial materials (~soil cap)

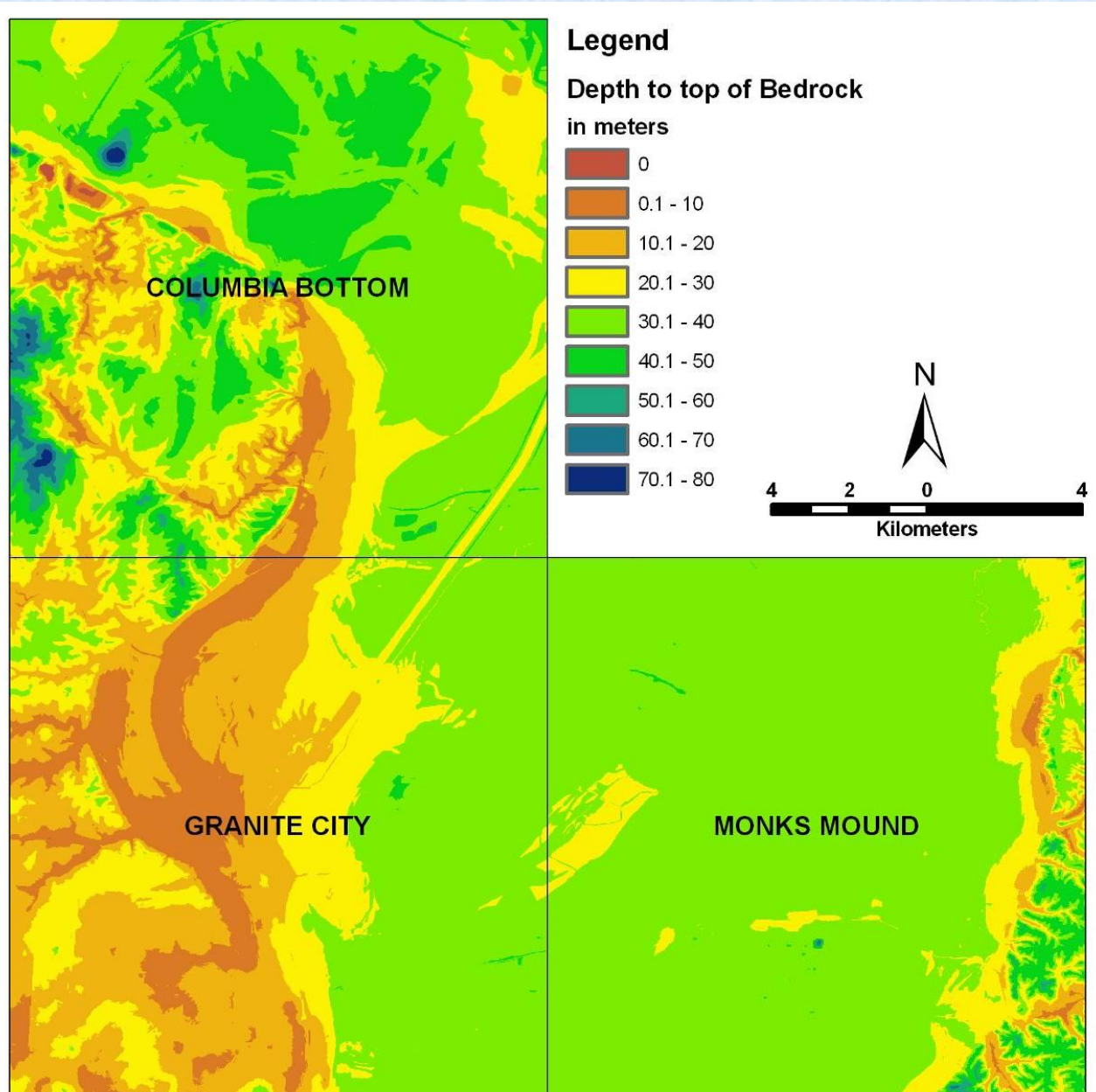
Physical soil properties

Dynamic soil properties (shear modulus and damping, shear wave velocity)

Digital Elevation Model used in pilot study



Depth to Bedrock (Surficial Geology Thickness Map)

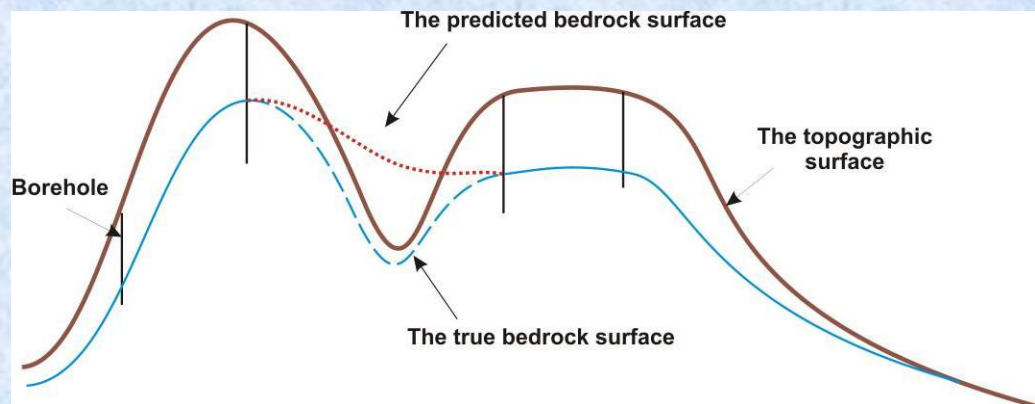
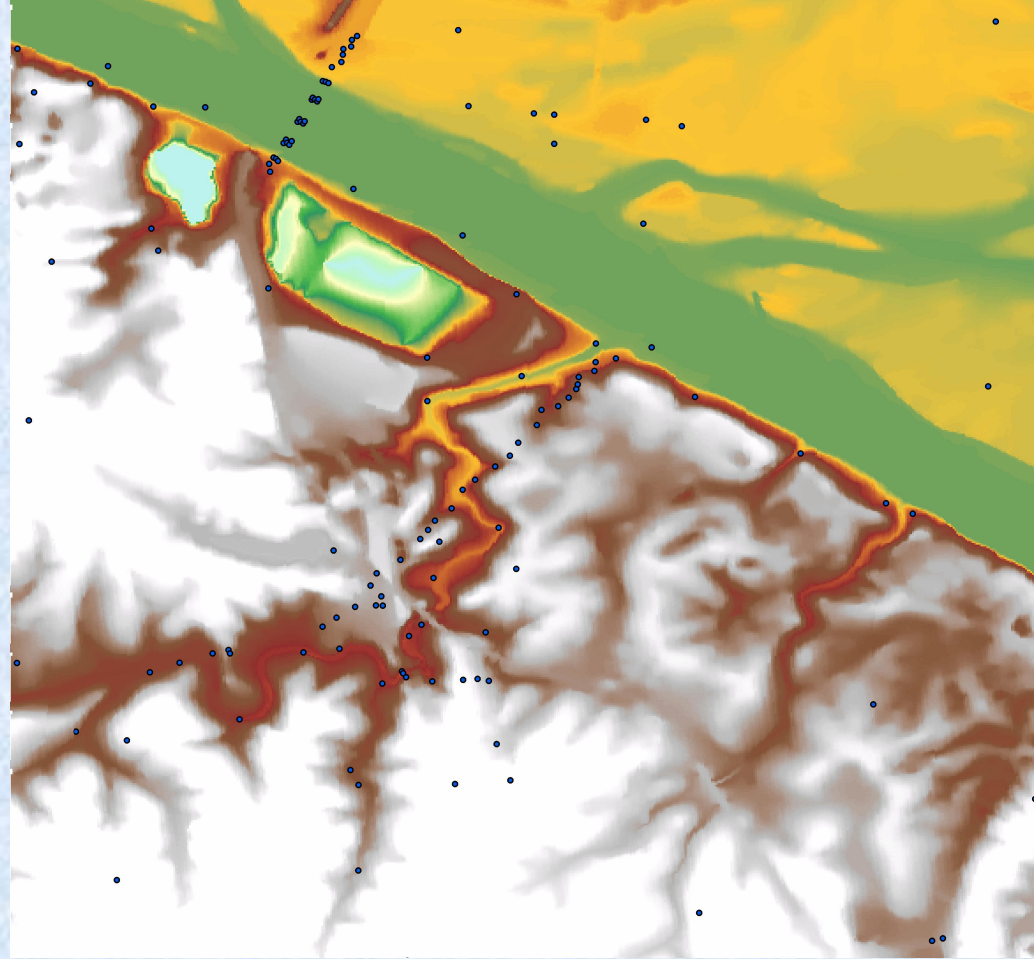


Drawbacks

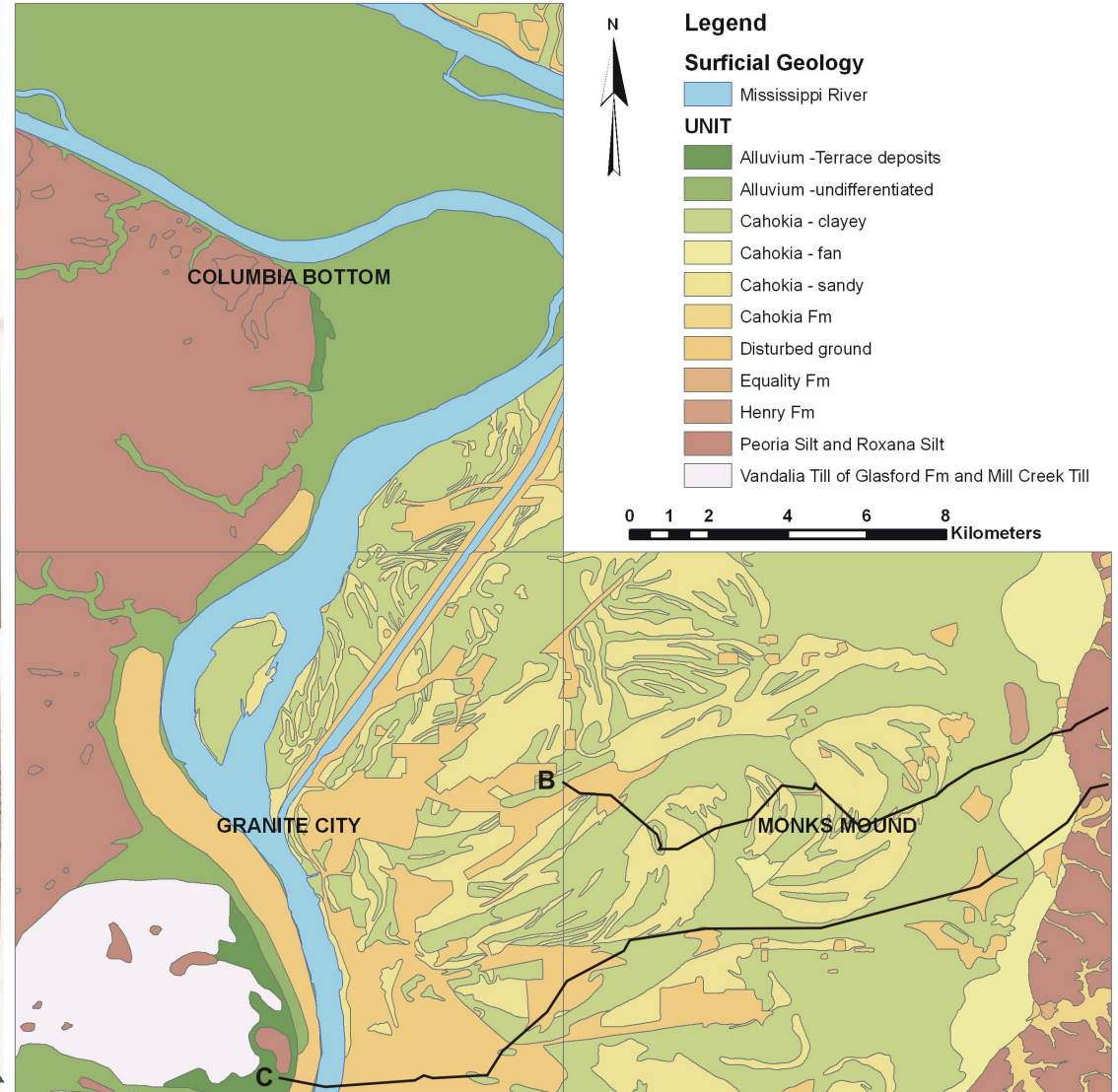
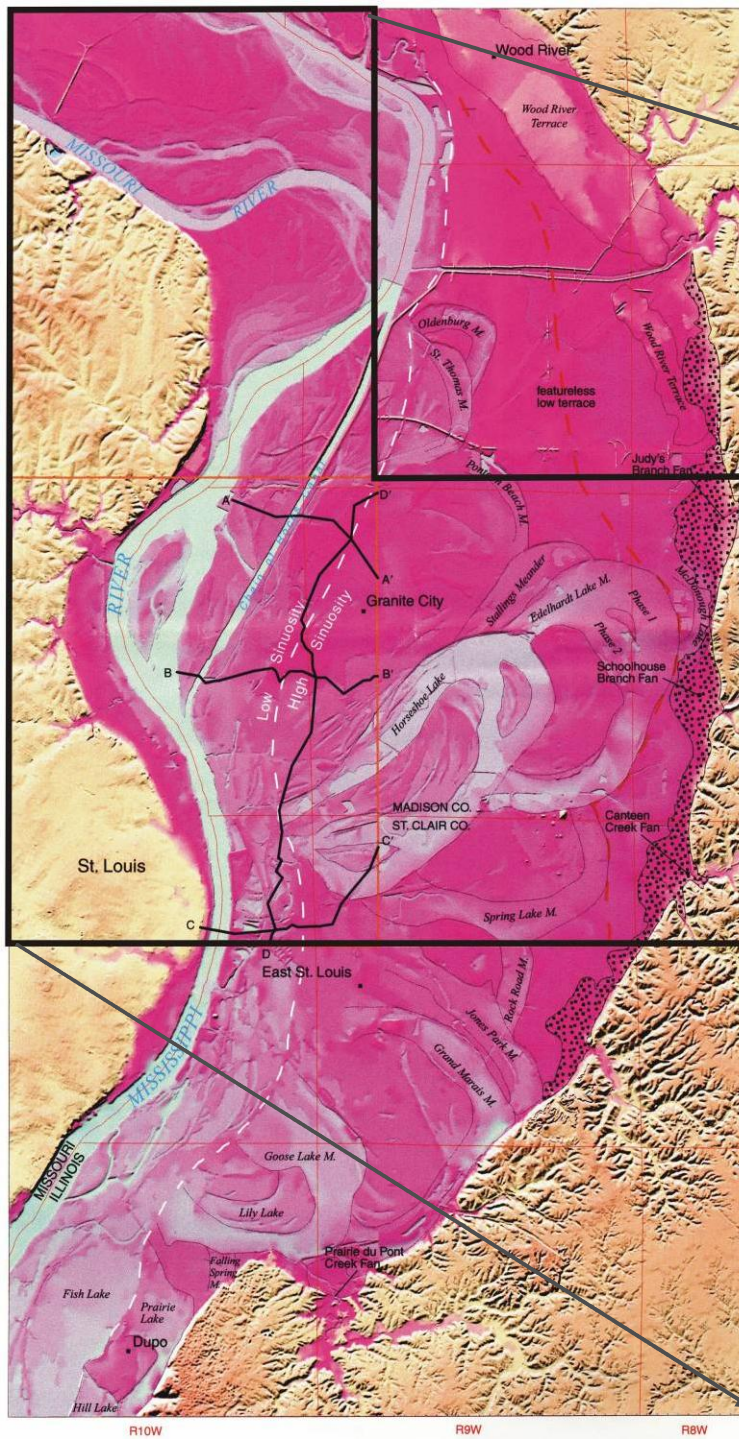
When the bedrock surface is uniform there is little uncertainty in the calculations. However, large variations in the data within small distances make predictions less certain.

The loess deposits mantling the uplands tend to thicken towards hilltops and thin towards valleys, because of erosion.

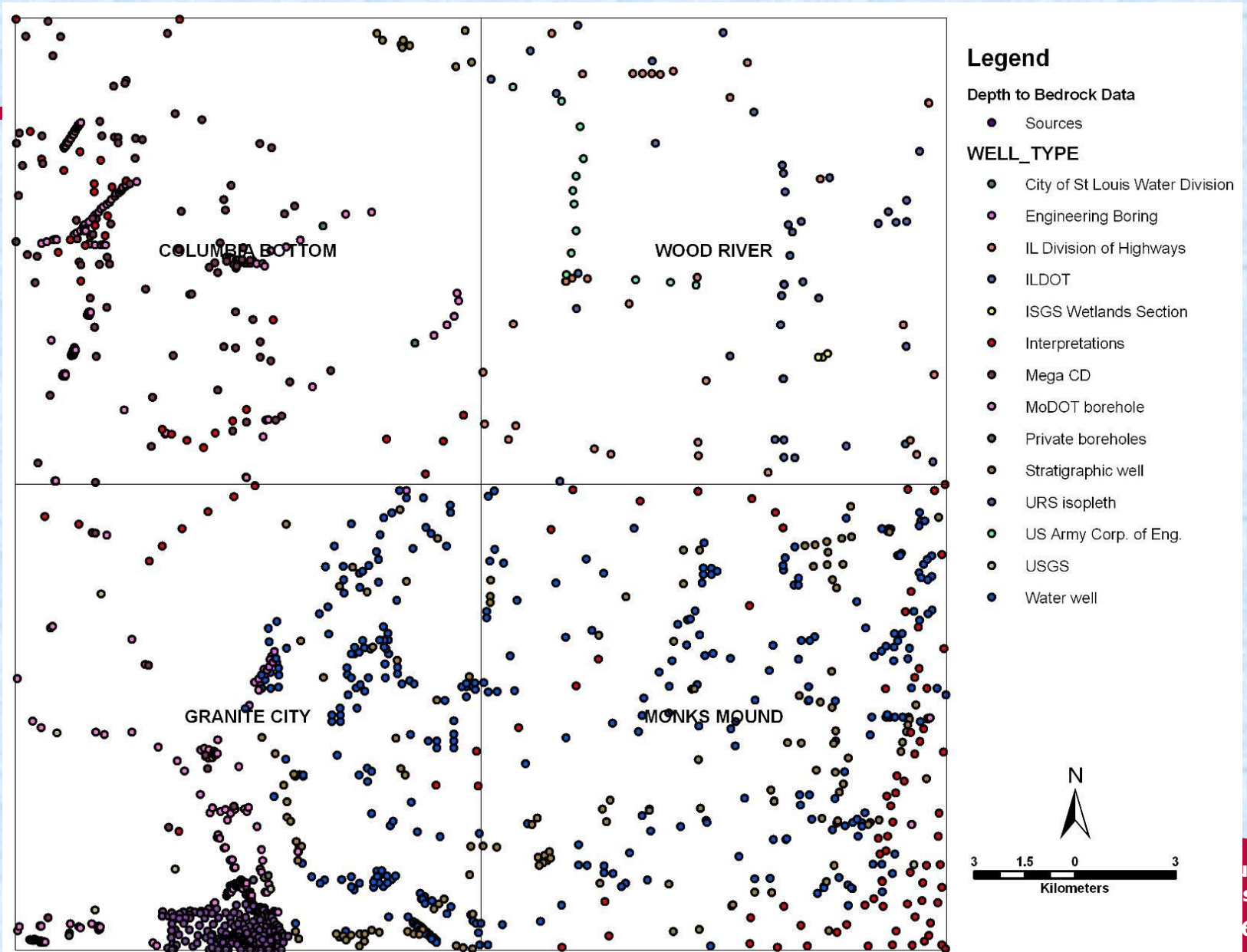
When thickness data is missing in these valleys, kriging techniques can be unreliable, as shown at lower right.



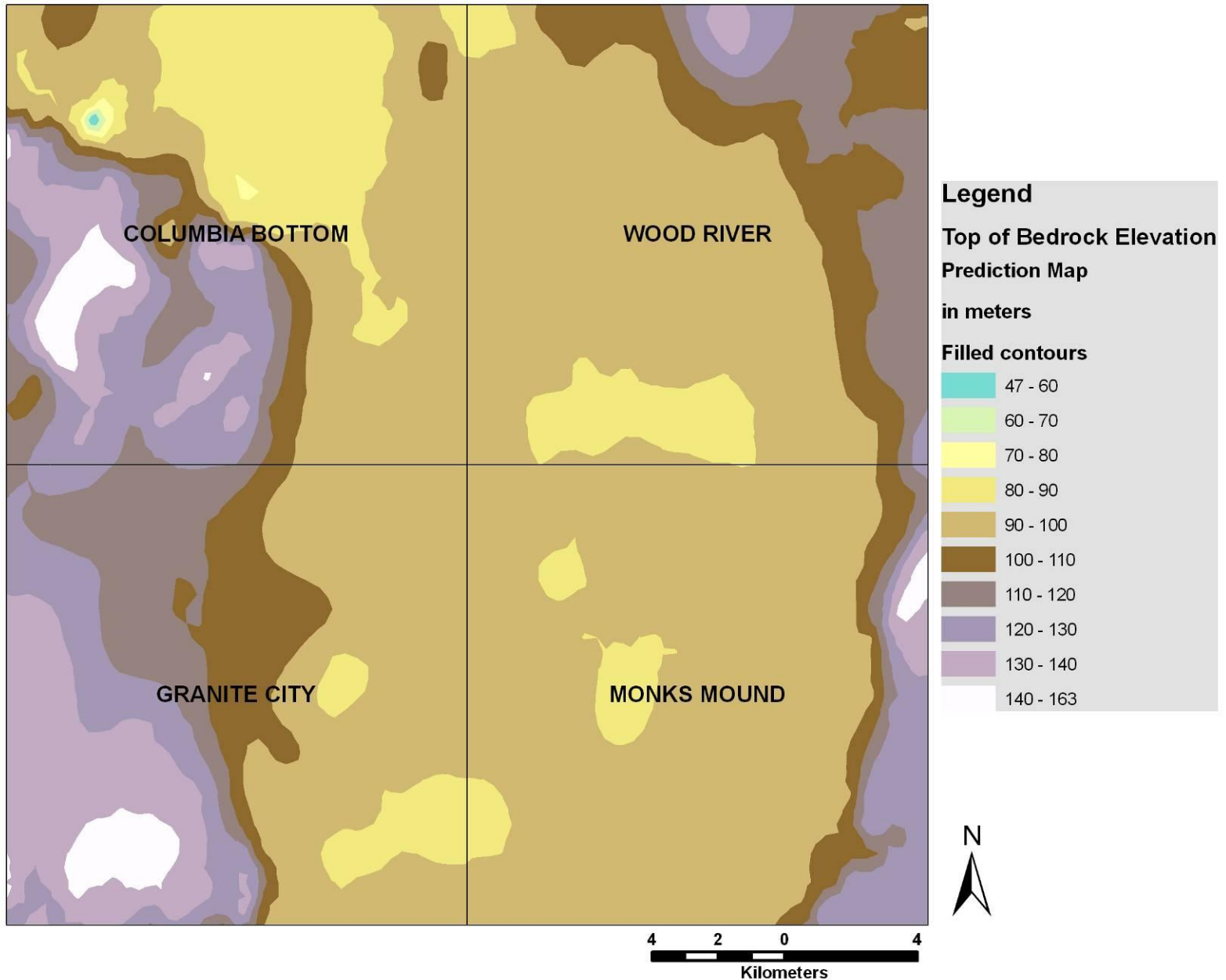
Surficial Geology of St. Louis study area



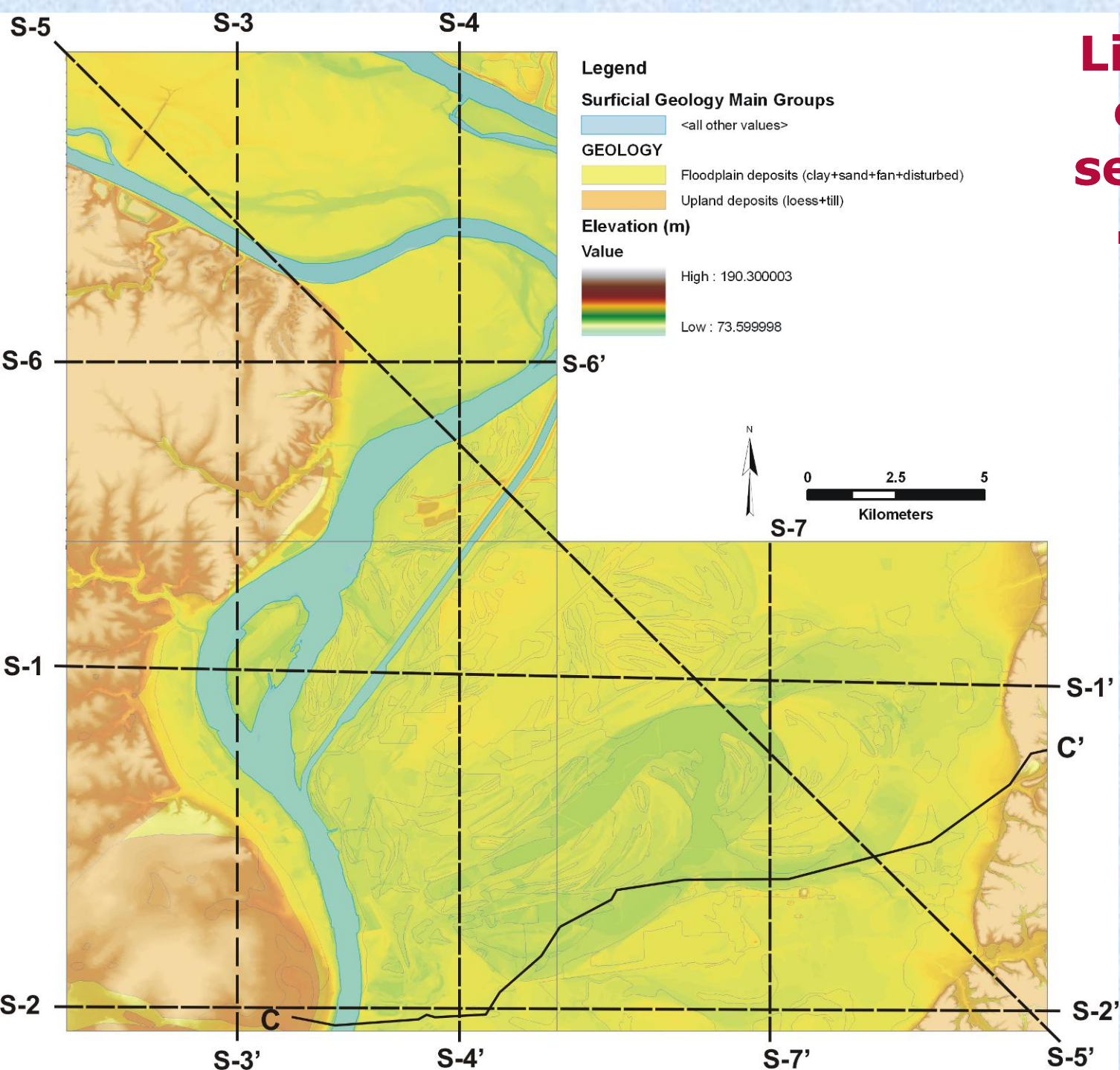
Boreholes Used in the pilot study



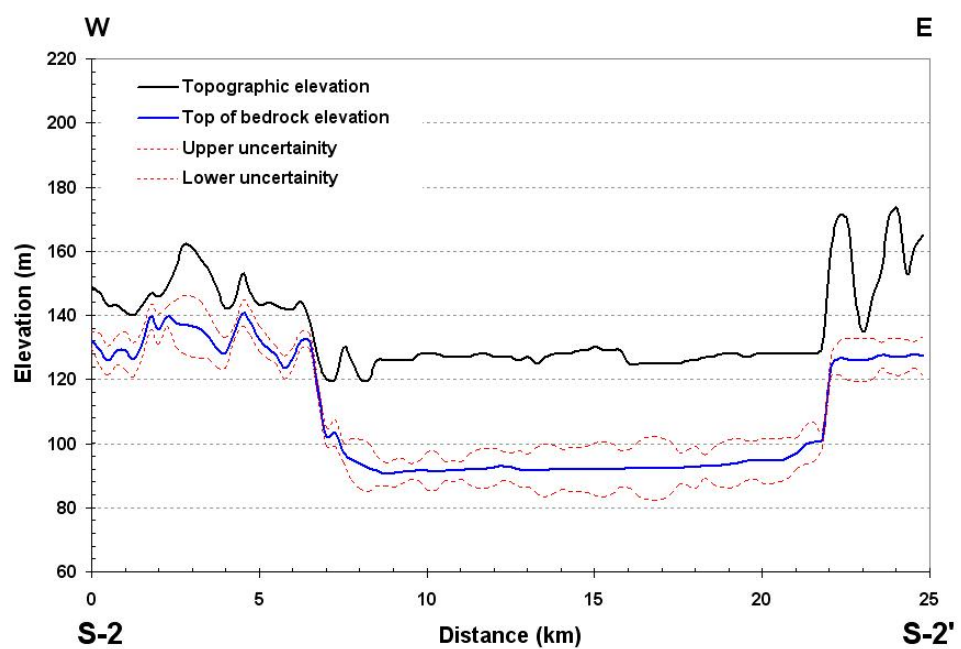
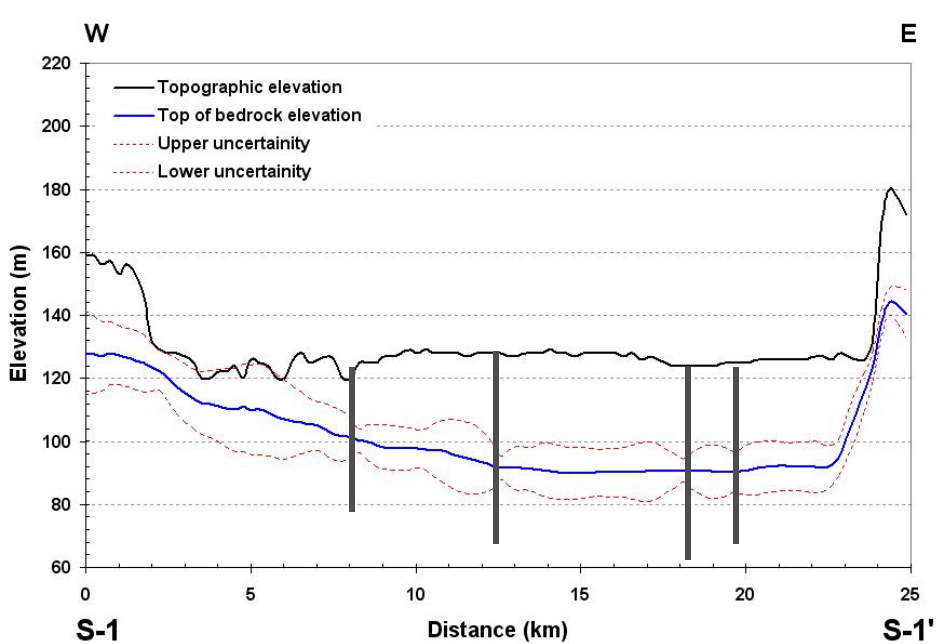
Estimation of Top-of-Bedrock Elevations



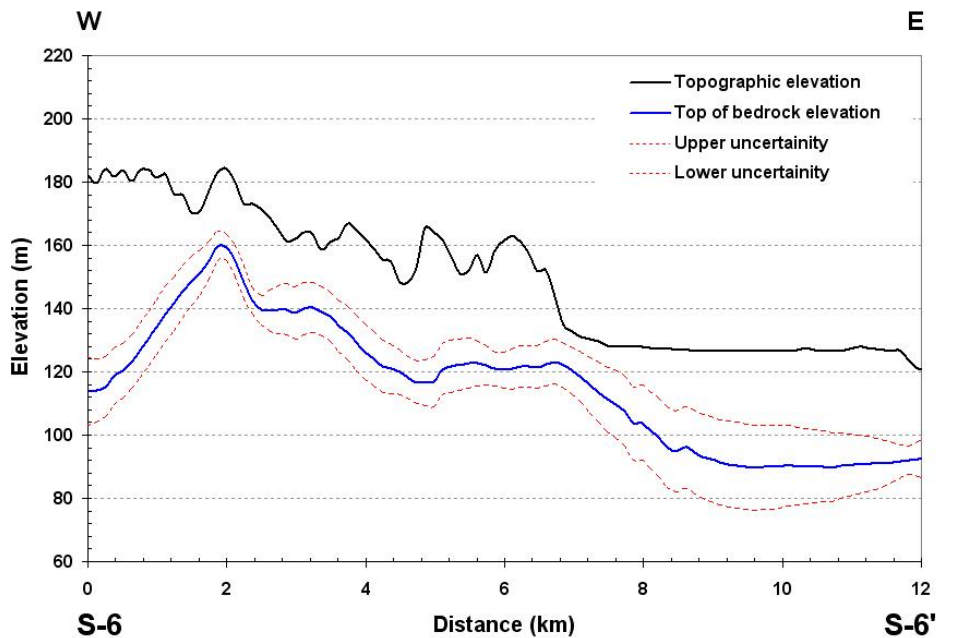
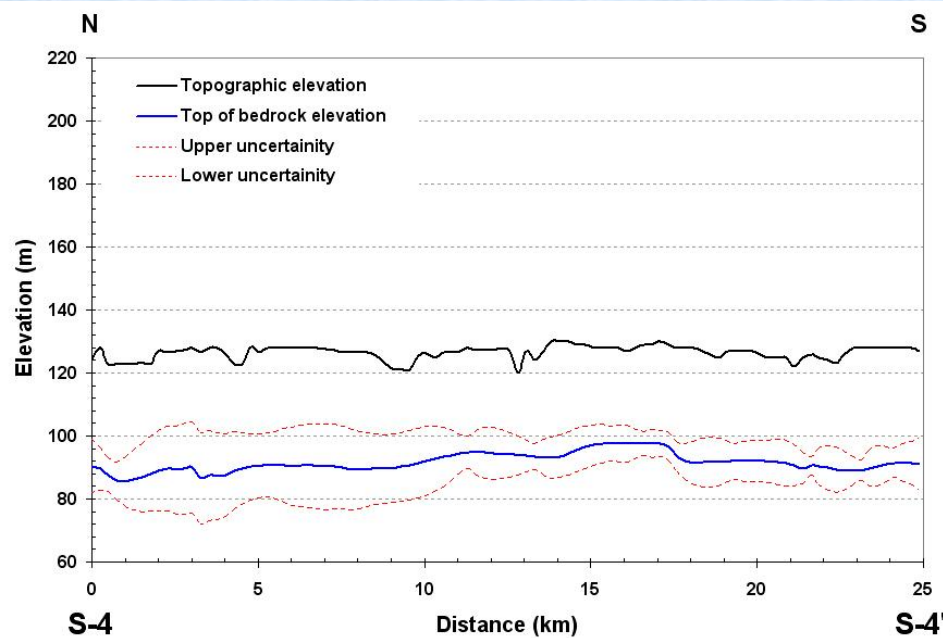
Lines of cross sections



Earthquakes
Mean Business
February 6, 2009



Cross sections with estimates of uncertainty

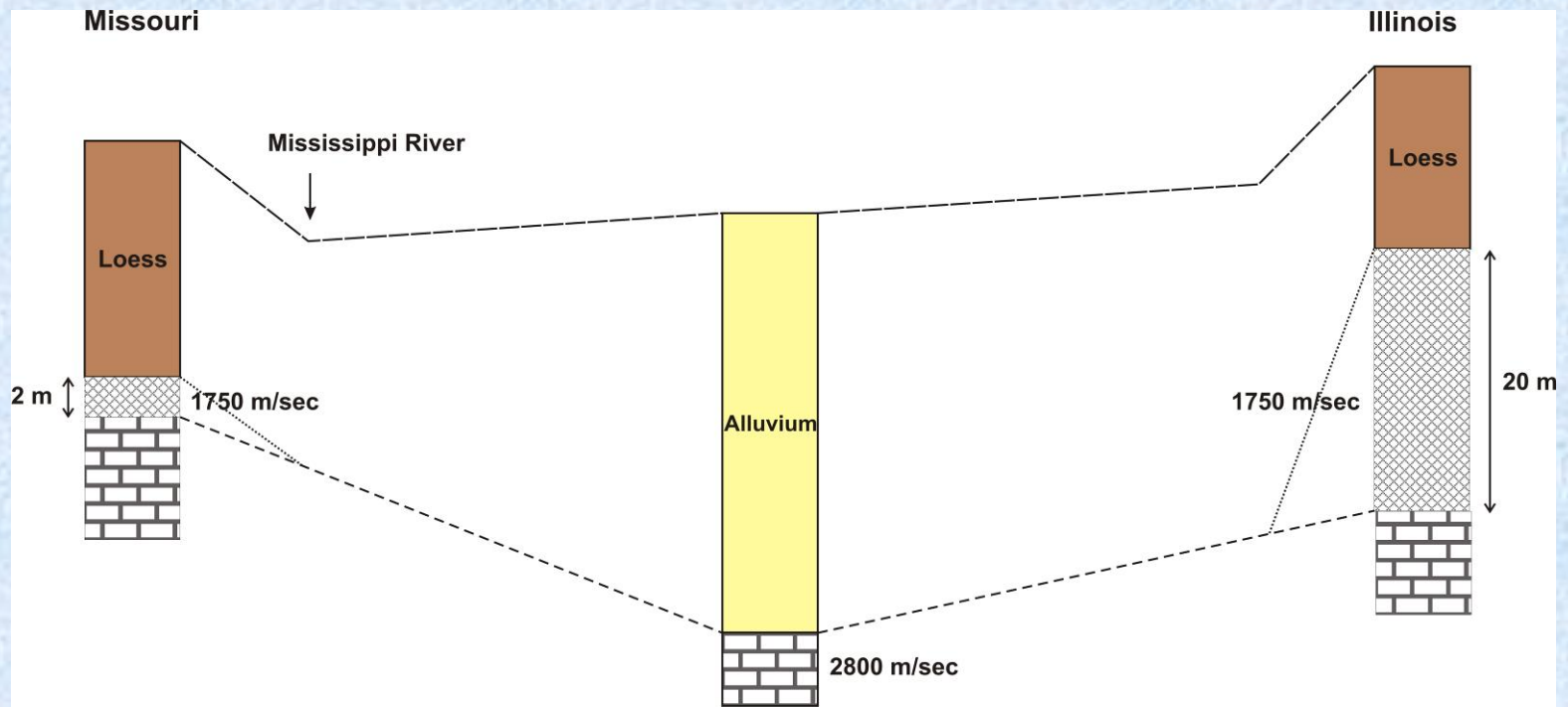


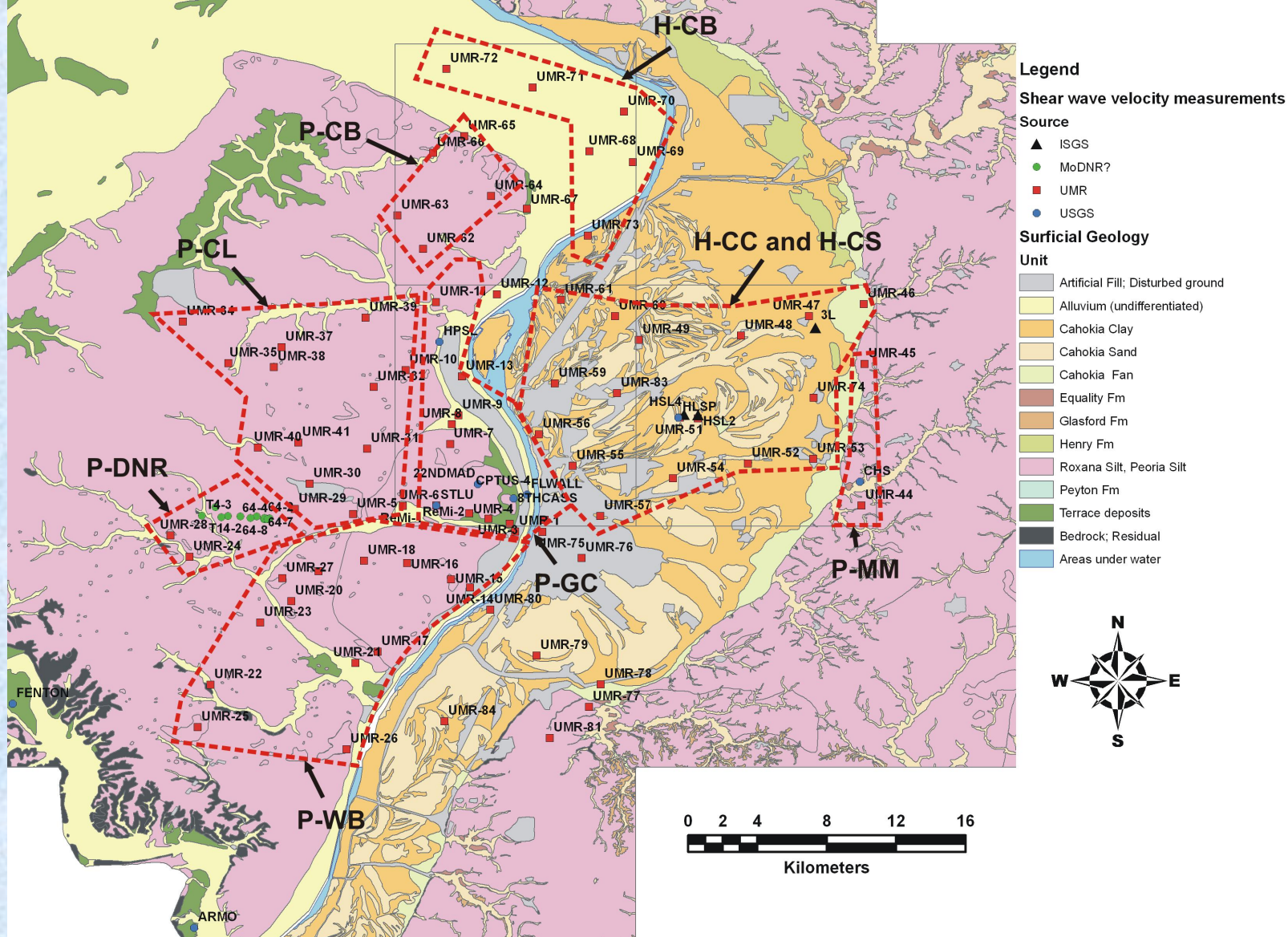
Bedrock properties

We used **1750 m/sec** +/- **250 m/sec** for the weathered bedrock shear-wave velocity, suggested by seismologist Robert Herrmann at St. Louis University.

We selected **0m / 2m / 20 m** thicknesses for the weathered bedrock.

We also used **2800 m/sec** for the half-space below the weathered bedrock.

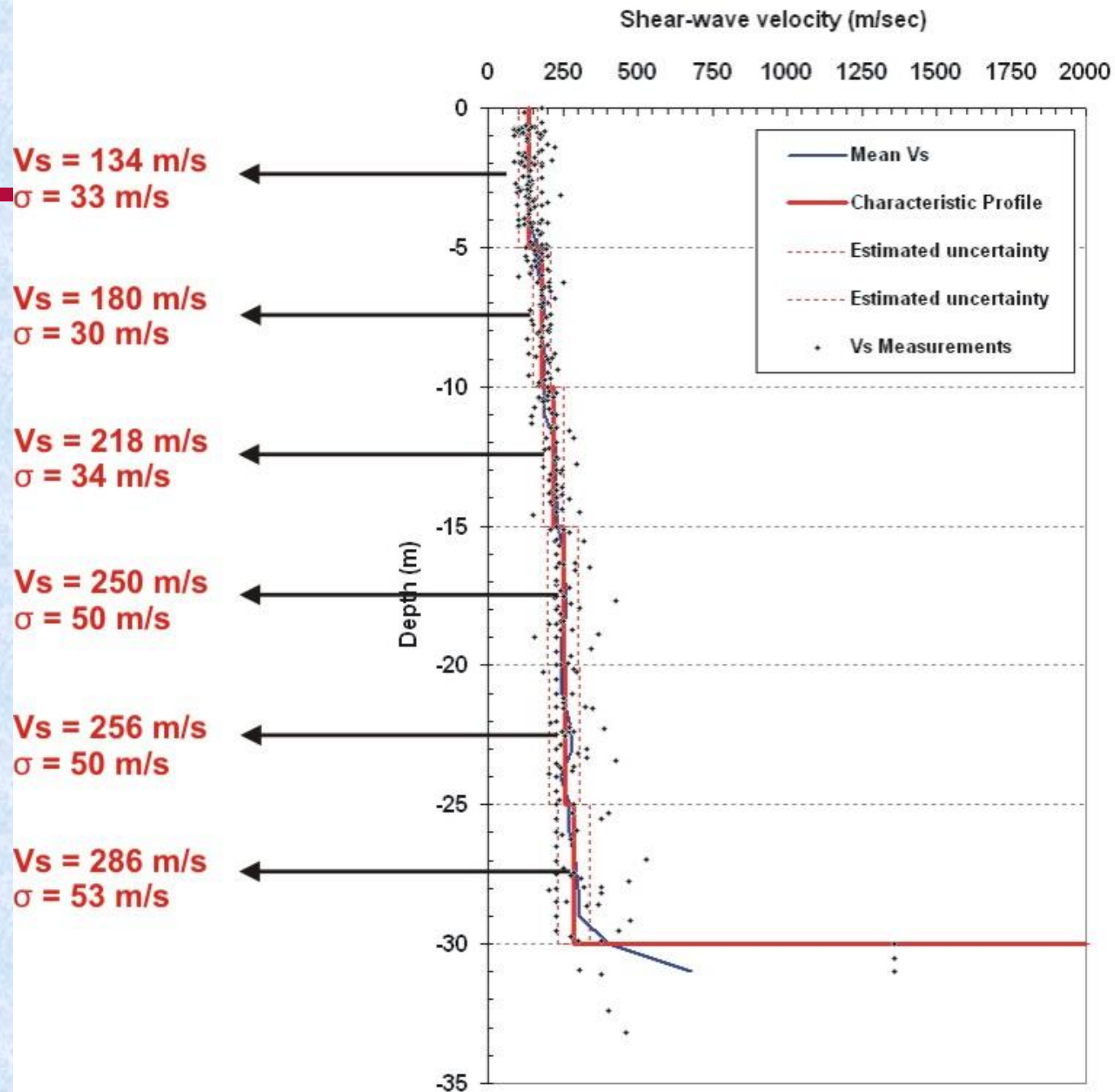




Characteristic V_s profiles were developed for nine geologic/geomorphic terrains, such as alluvial or loess/colluvial covered uplands, etc.

Characteristic Vs Profiles

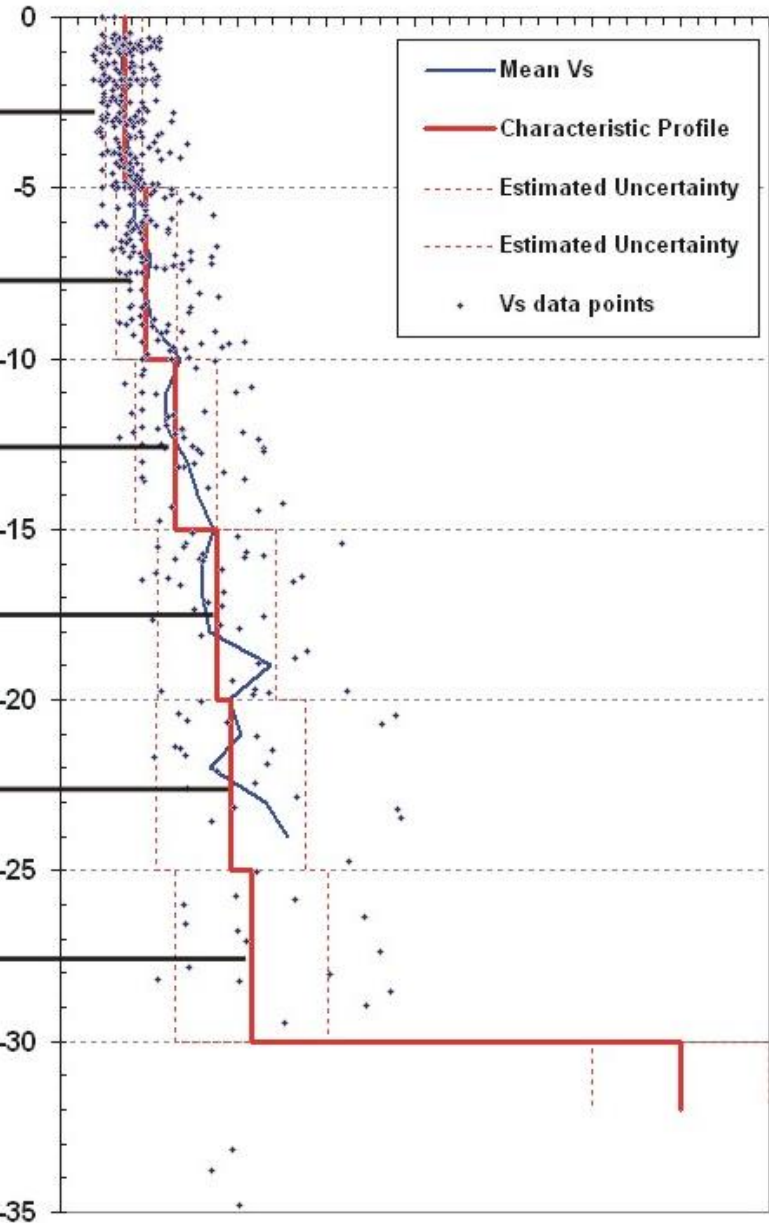
**Floodplain
(Alluvial)
deposits**



Characteristic Vs Profiles

Shear wave velocity (m/sec)

0 250 500 750 1000 1250 1500 1750 2000



Vs = 179 m/s
 σ = 51 m/s

Vs = 241 m/s
 σ = 86 m/s

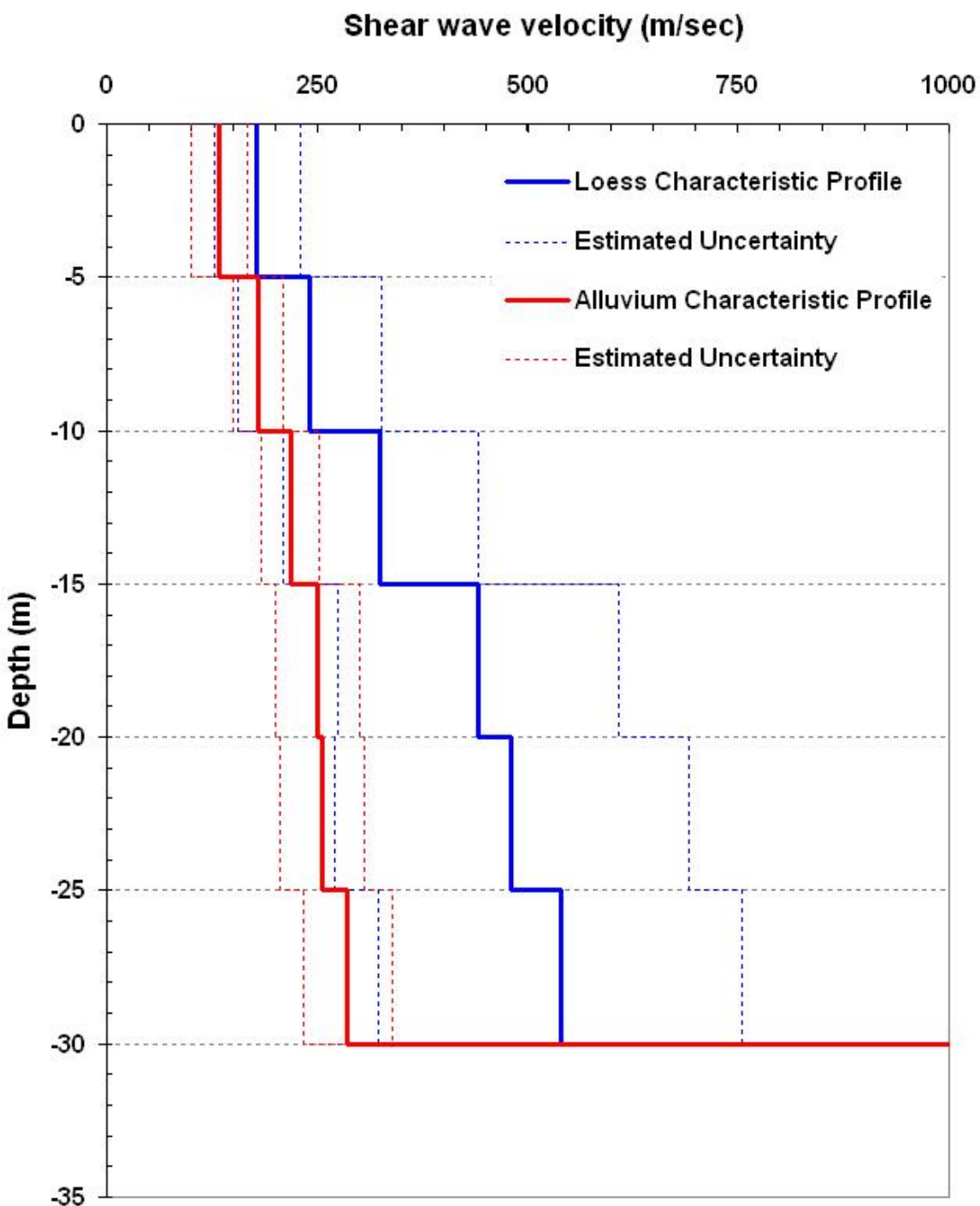
Vs = 325 m/s
 σ = 116 m/s

Vs = 442 m/s
 σ = 167 m/s

Vs = 481 m/s
 σ = 211 m/s

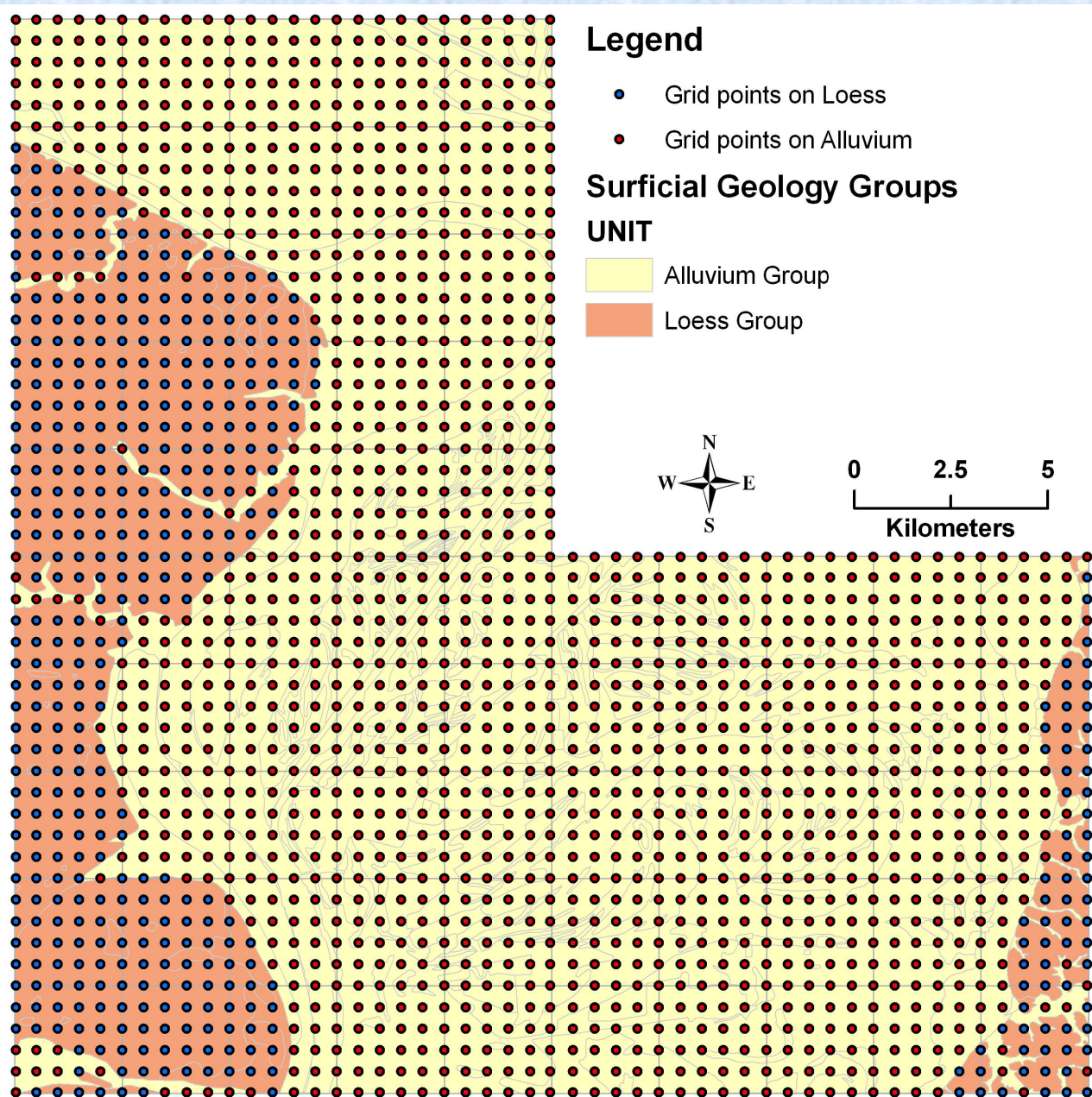
Vs = 539 m/s
 σ = 217 m/s

**Loess-
covered
Upland
deposits**



**Comparisons
between Vs
profiles for
Alluvium in the
major river
valleys and
the Loess
covered uplands**

Amplification Calculation Procedure



Total of **1,974** grid points, 500 m apart

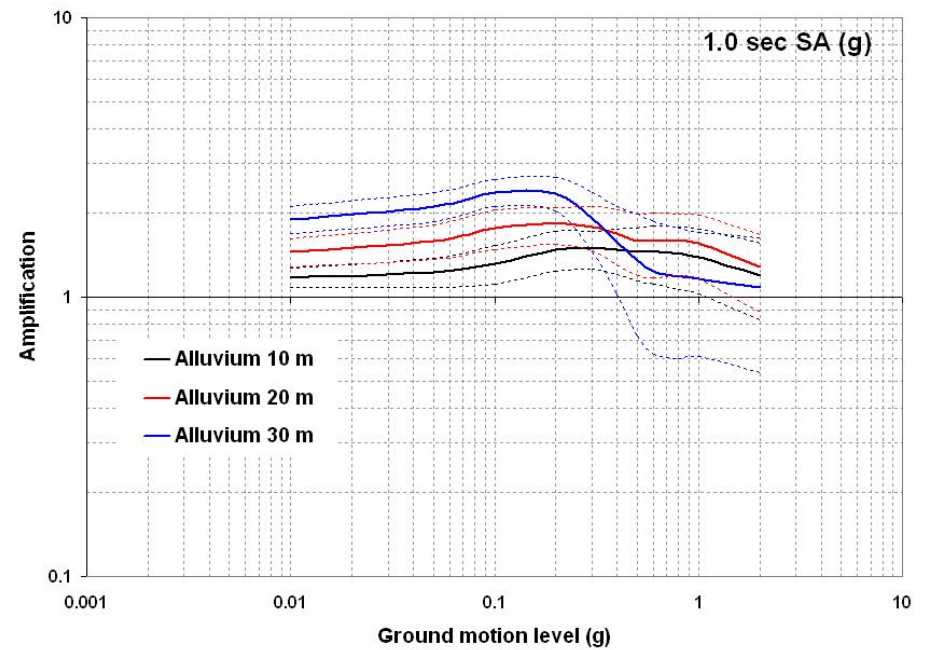
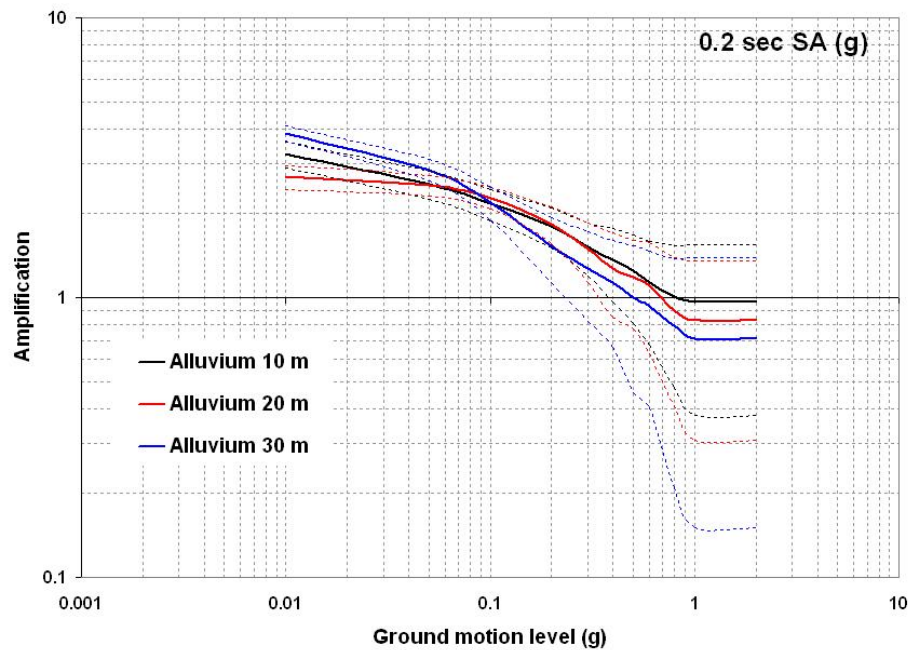
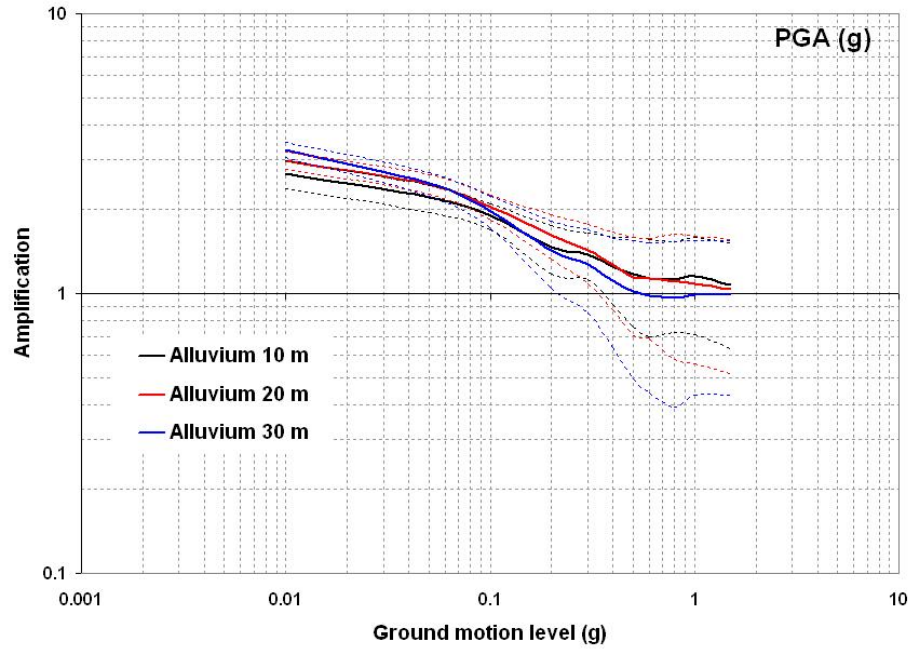
For every grid point, calculations were performed 100 times for the 10 ground-motion levels and three ground motion parameters (PGA, 0.2sec Sa, and 1 sec Sa), bringing the total to 3000 calculations per grid point.

When multiplied to the total number of grid points, more than **5,400,000** calculations were made.

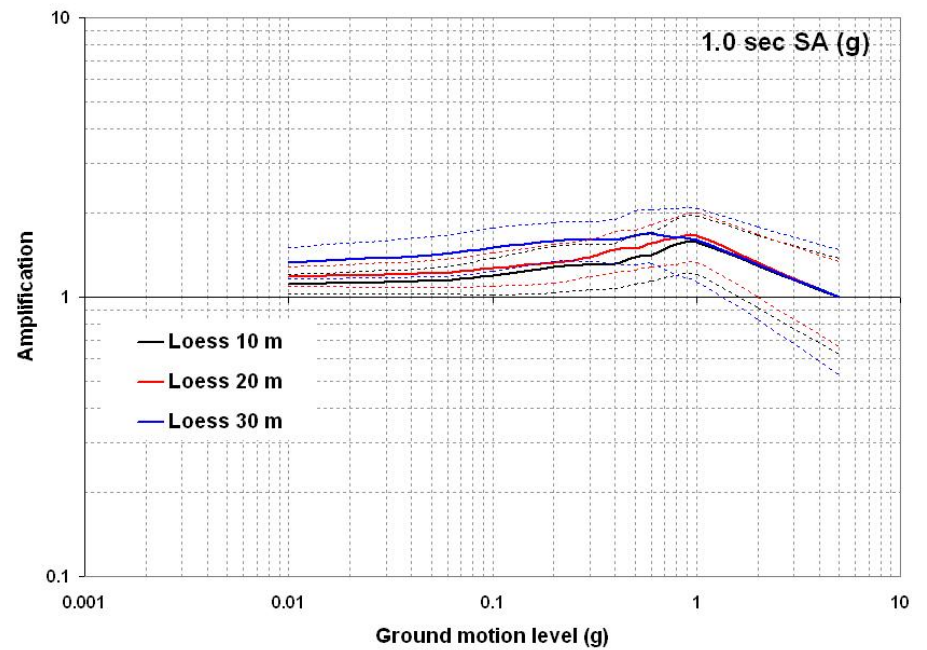
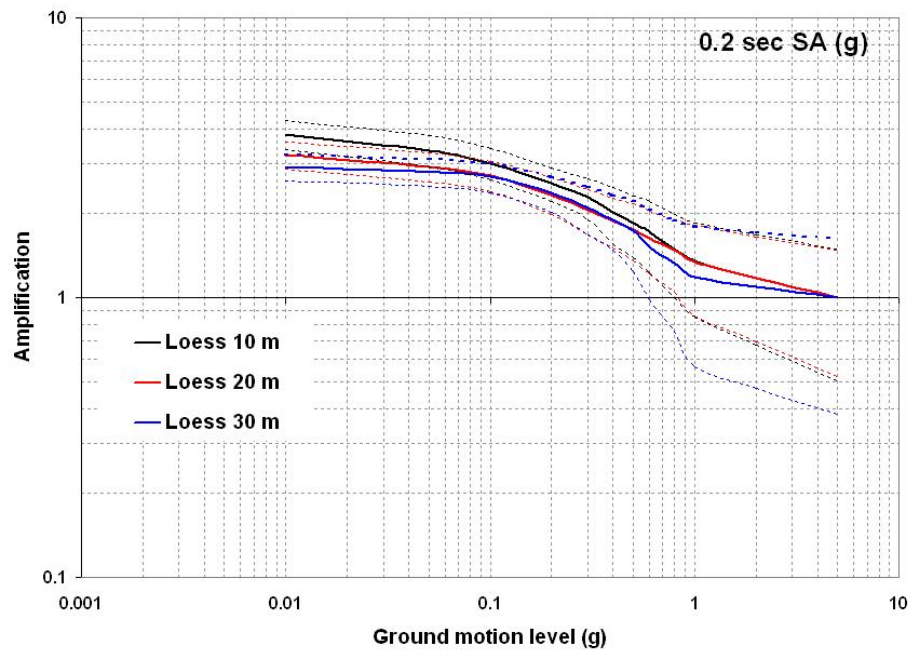
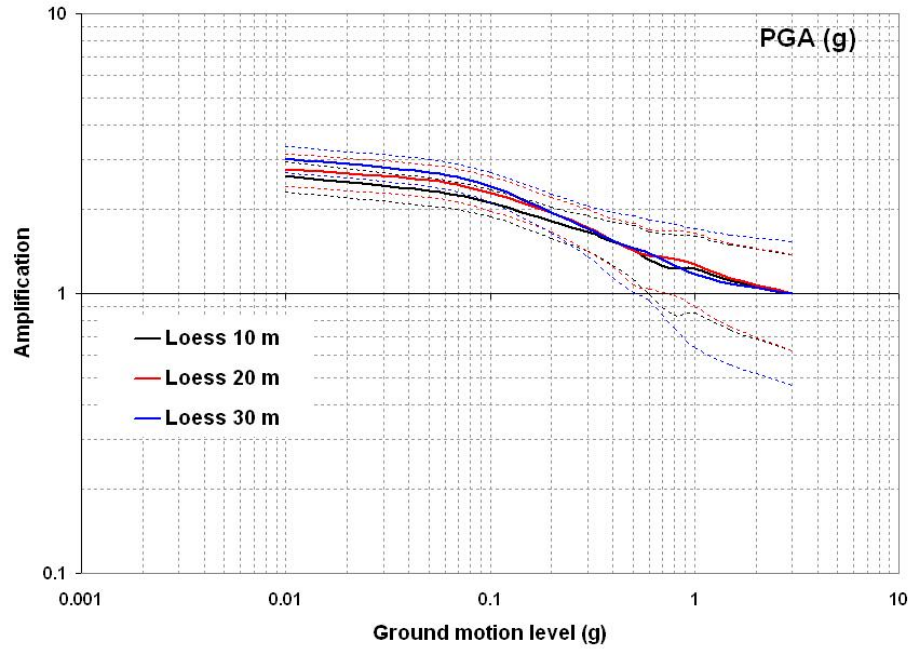
Step 6

Distribution of Site Amplification

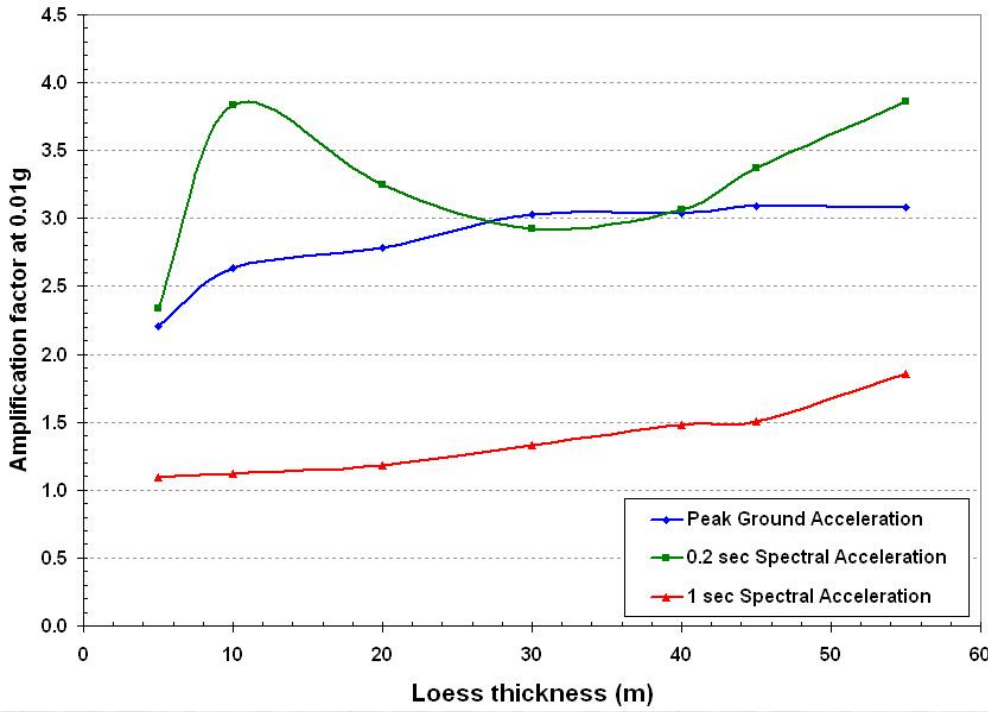
Distribution of Site Amplification in Alluvium



Distribution of Site Amplification in Loess

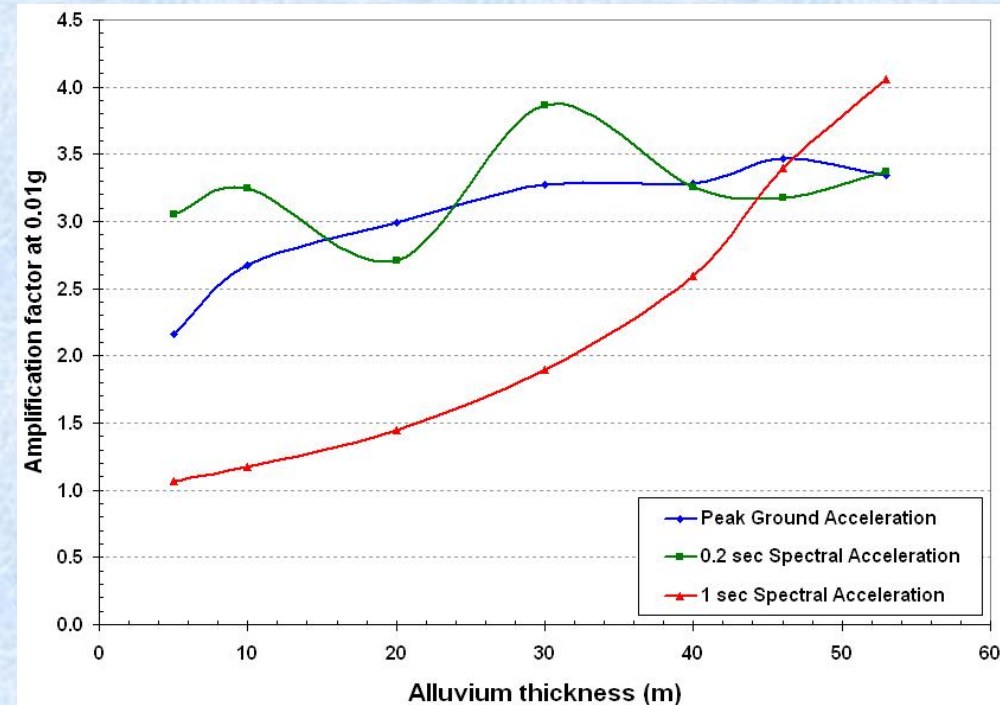


Soil Cap Thickness vs. Ground motion



Upland Profiles

Floodplain Profiles



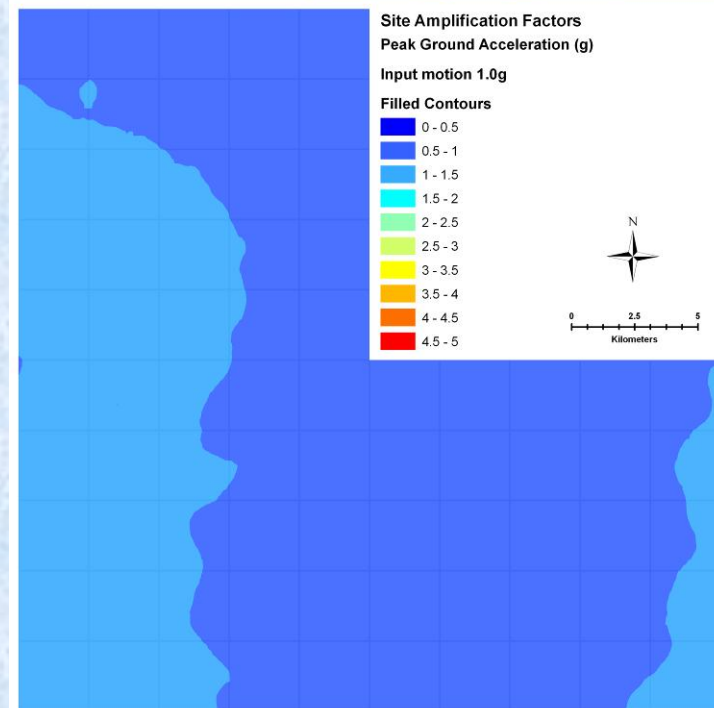
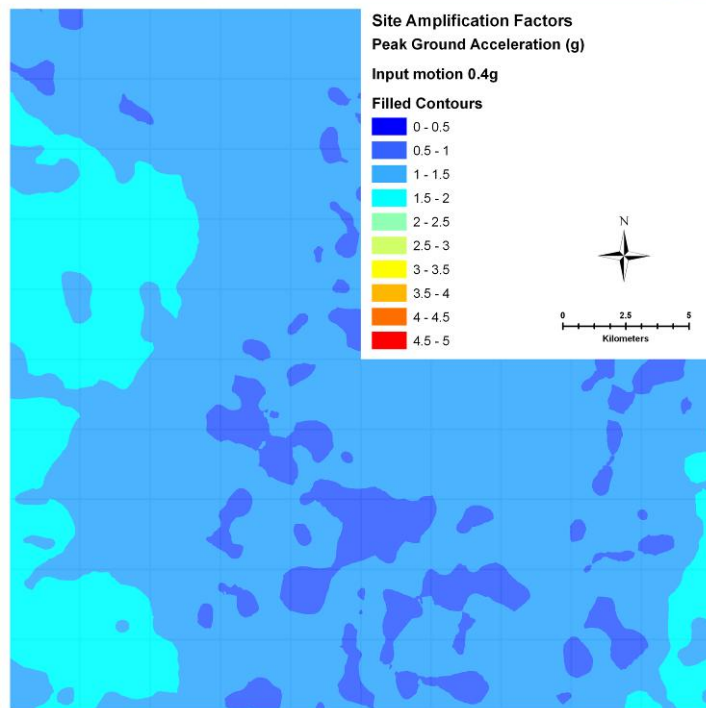
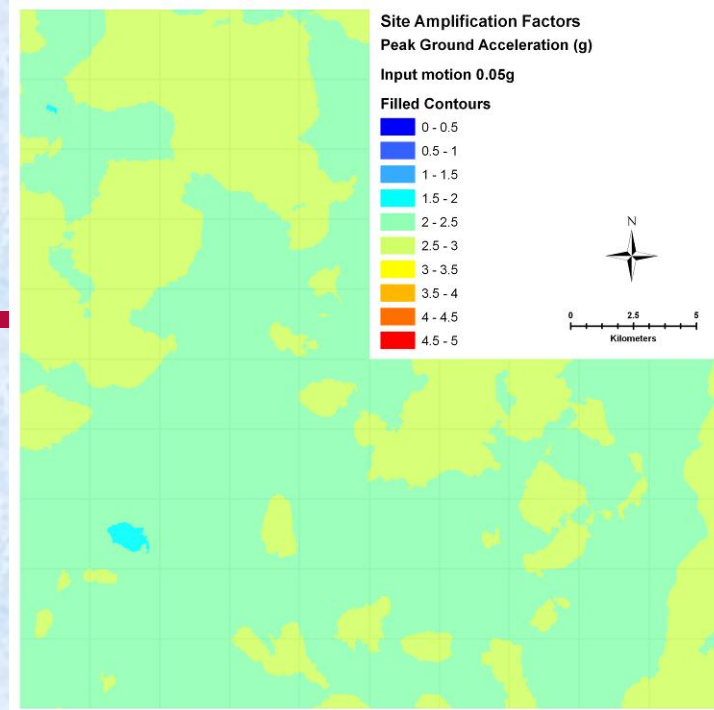
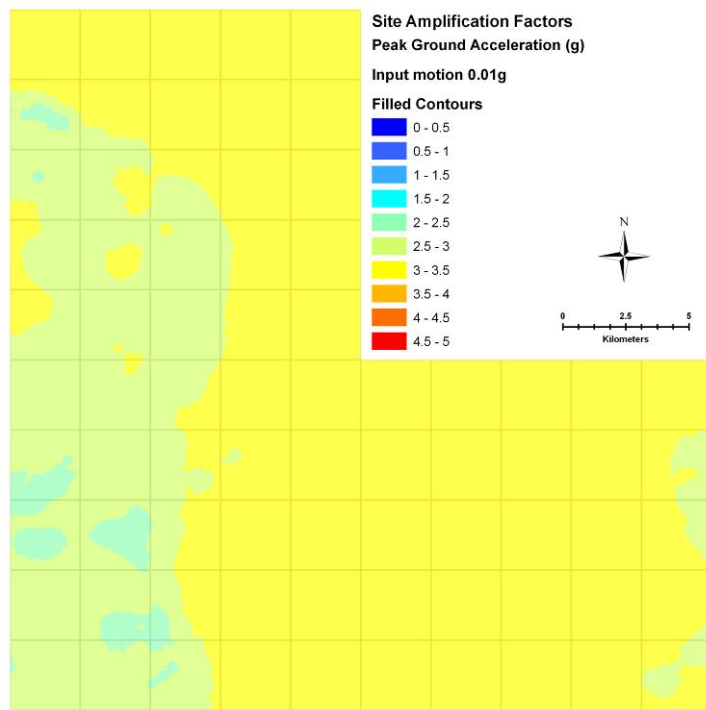
Step 7

Site Amplification Maps

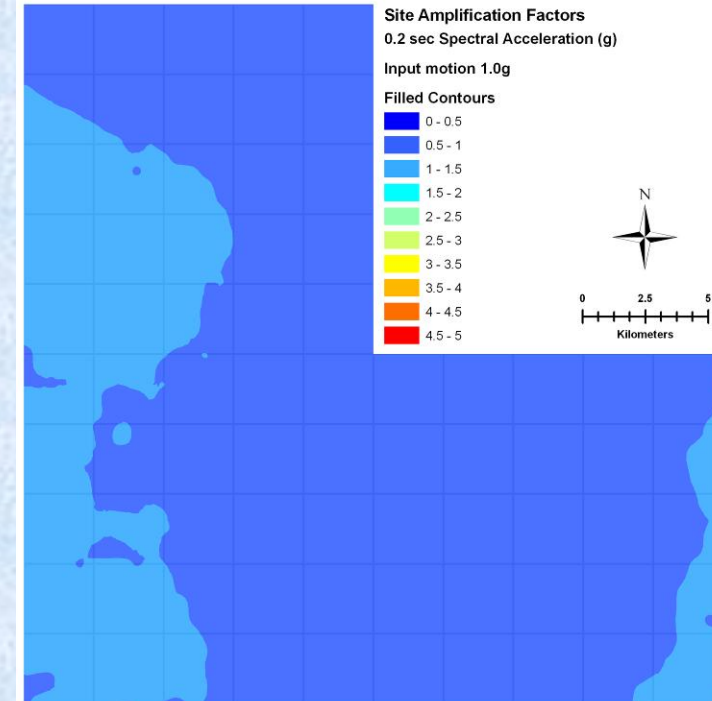
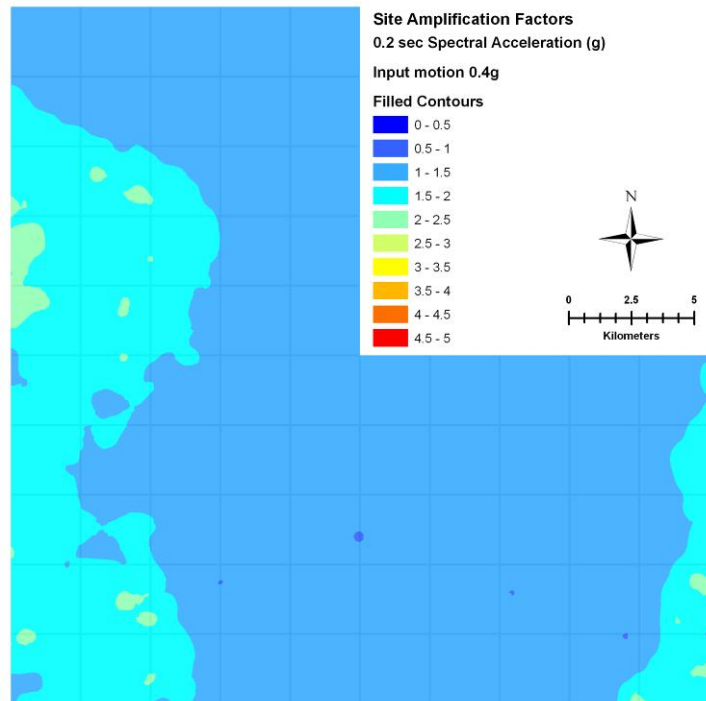
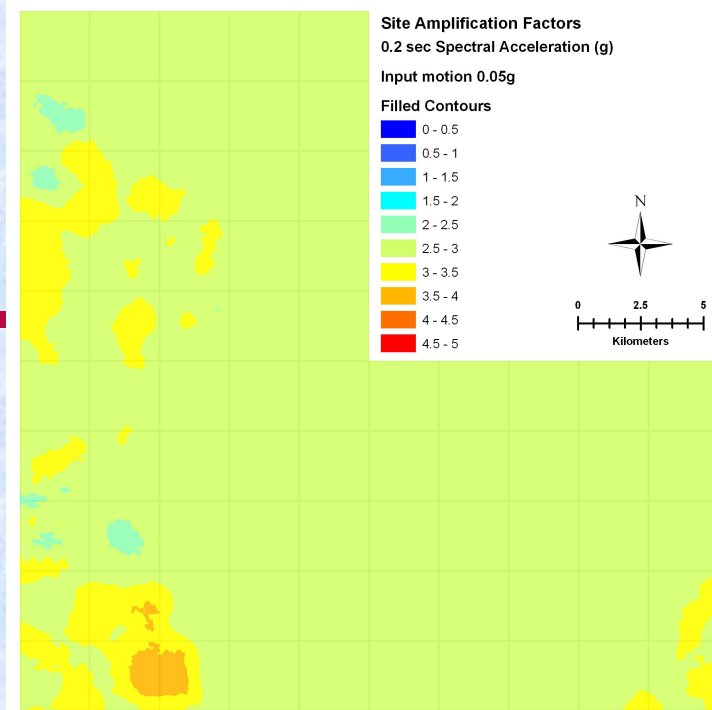
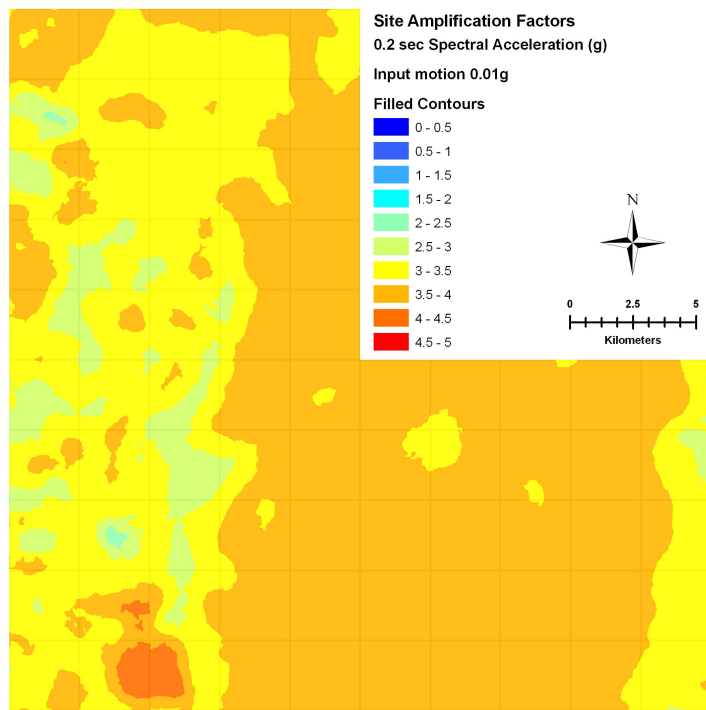
Site amplification maps were generated for discrete increments of ground motion (0.01 to 1.0 g) and for the following ground motion parameters:

- **Peak Ground Acceleration (PGA)**
- **0.2 second Spectral Acceleration**
- **1.0 second Spectral Acceleration**

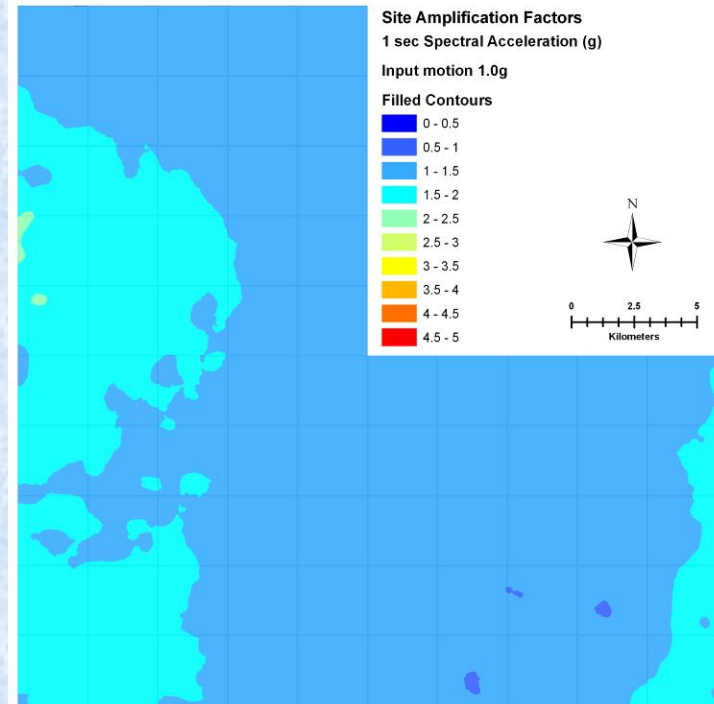
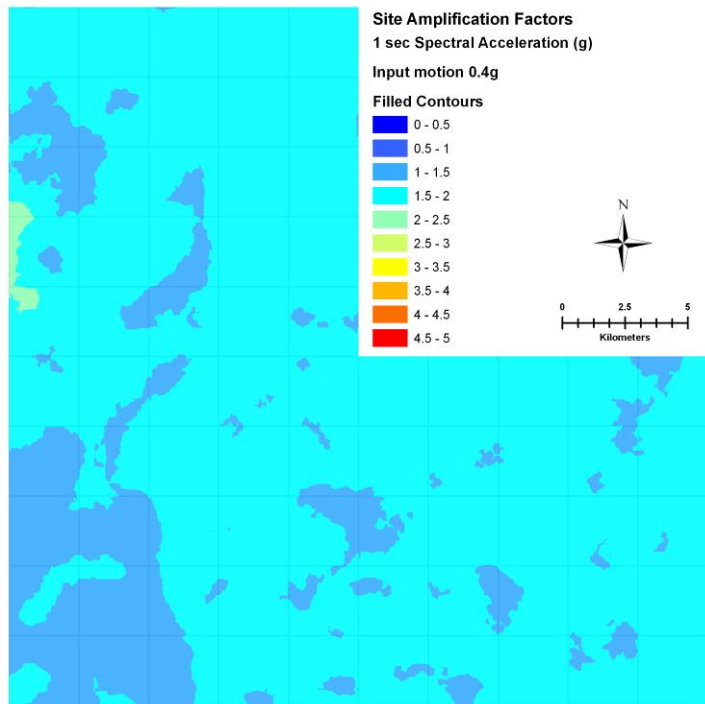
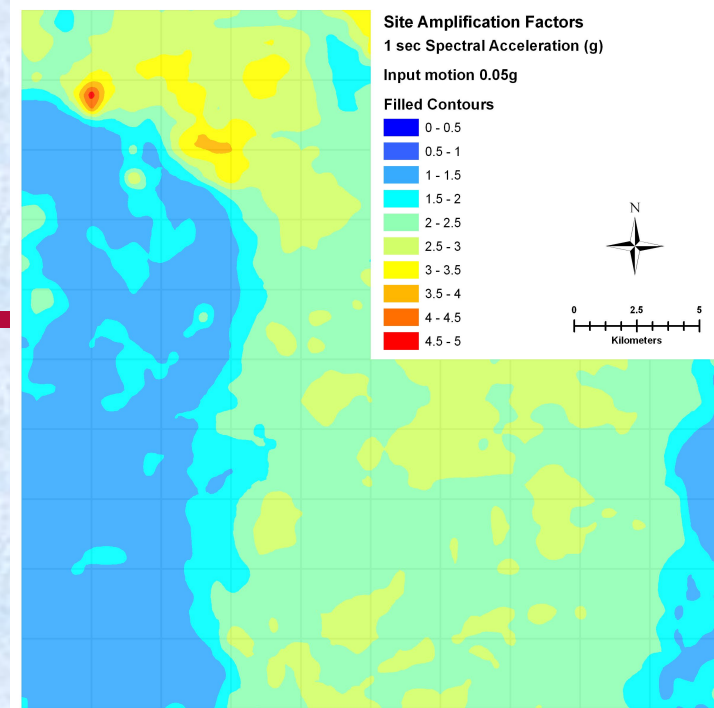
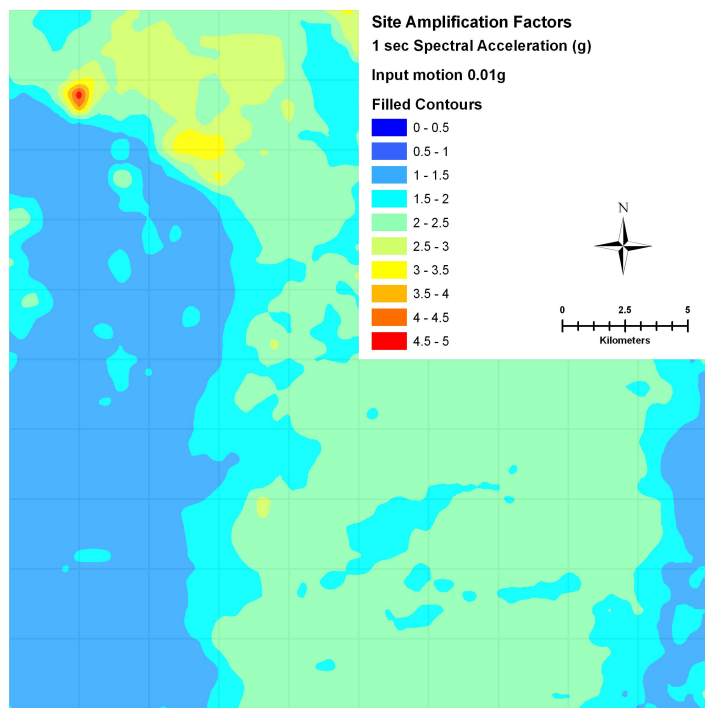
PGA (g)



0.2 sec SA



1.0 sec SA



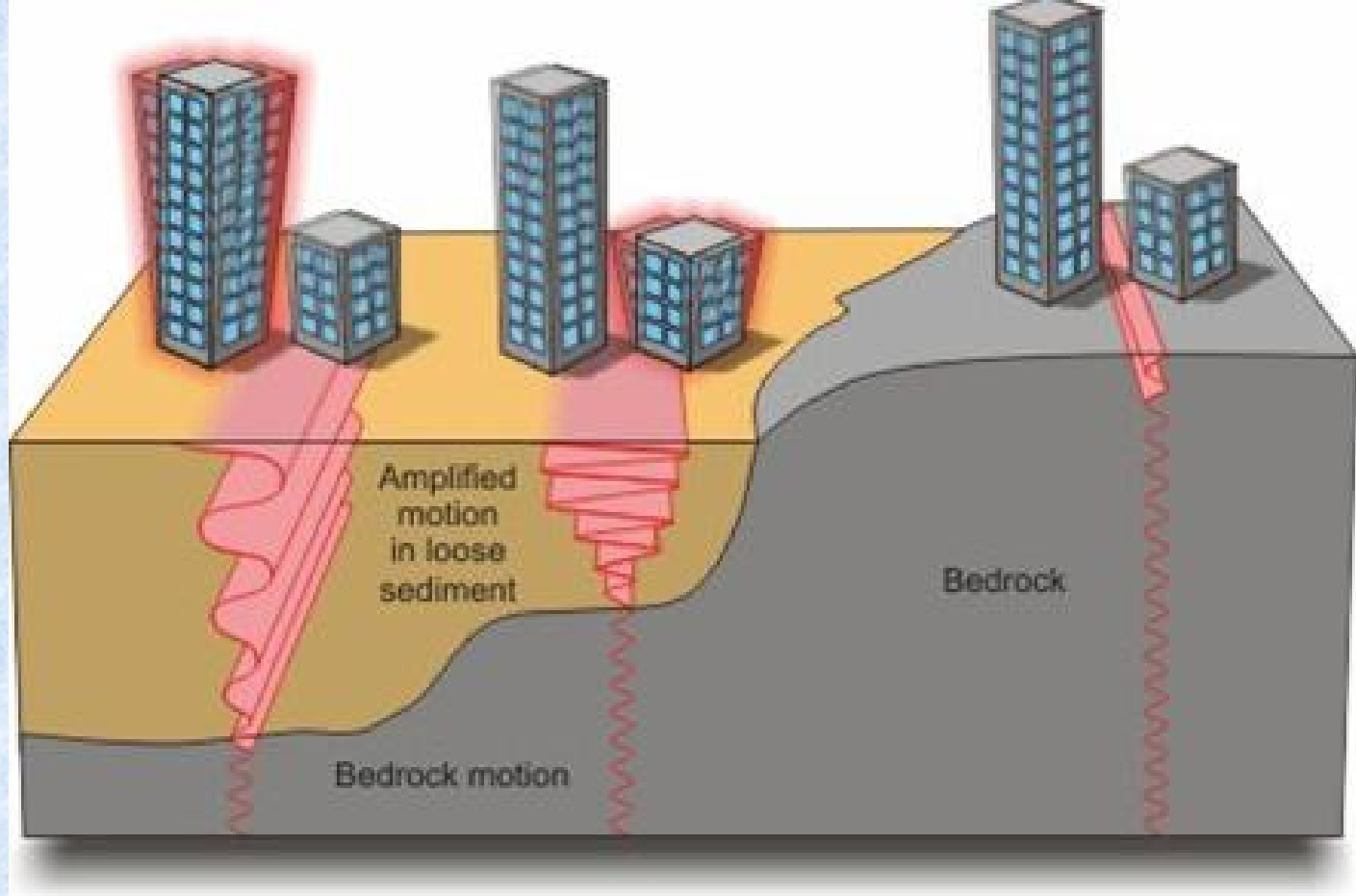
Summary of Results

Site amplification depends on the severity of the assumed input ground motion.

Site amplification also depends on the geologic conditions underlying any given location.

Site Amplification is severe on upland sites underlain by thick deposits of loess.

Site Amplification is also severe for long period structures on deep ($>\sim 20$ m) alluvial sites, in the major river flood plains.



Left – Deeper alluvial cover (~ 31 m) tends to magnify long period (SA 1.0 sec) motions

Middle – Medium alluvial cover (~ 18 m) tends to magnify motions for 0.2 sec SA

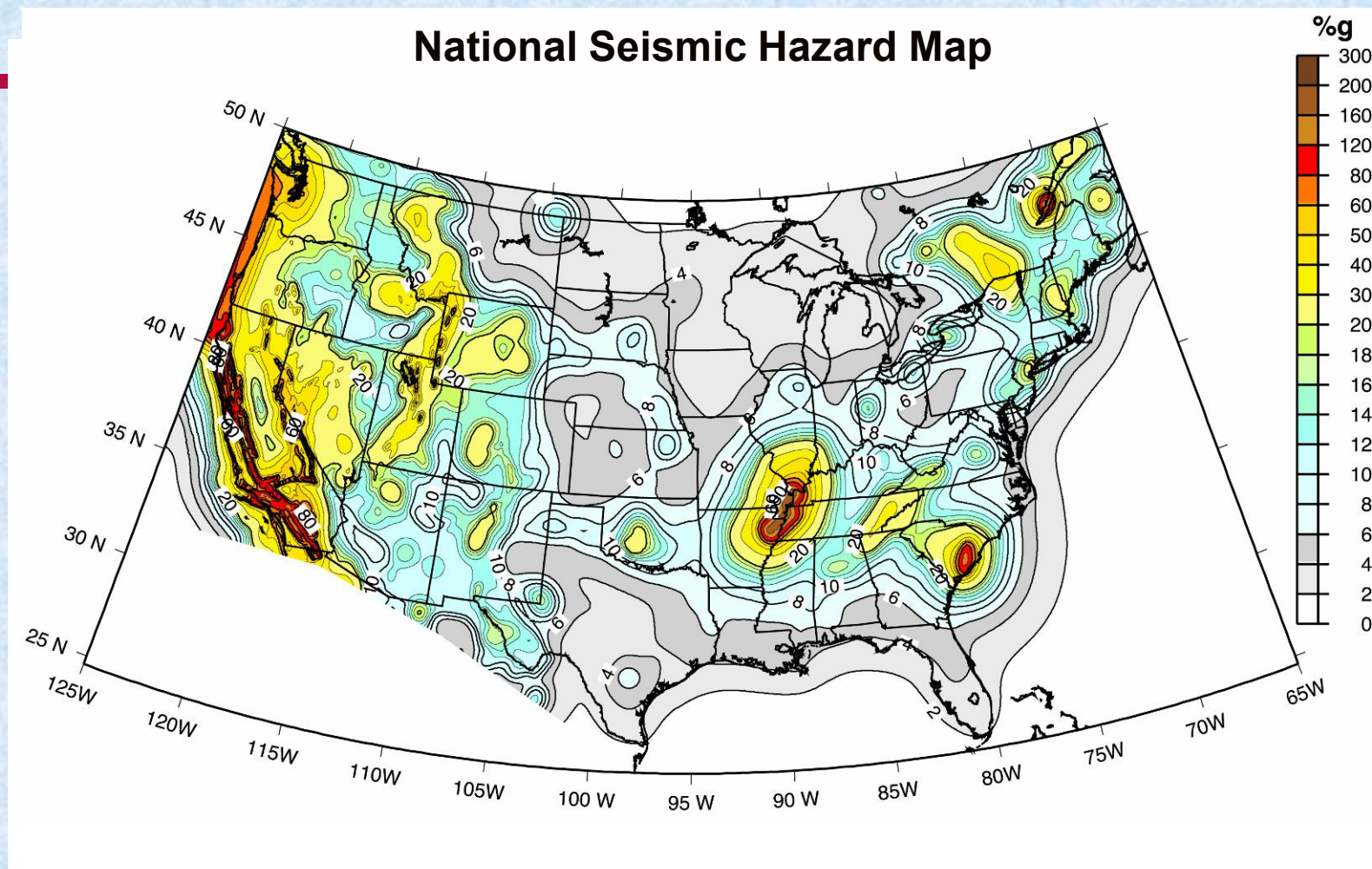
Right – Upland sites mantled by loess tend to magnify bedrock motion because of impedance contrast between bedrock and soil cap.

Seismic Hazard Maps

Previous Examples:

- **National Seismic Hazard Maps (2002)**
- **Memphis Shelby County Seismic Hazard Maps (2004)**

The National Seismic Hazard Maps were constructed using the best earth science information available.



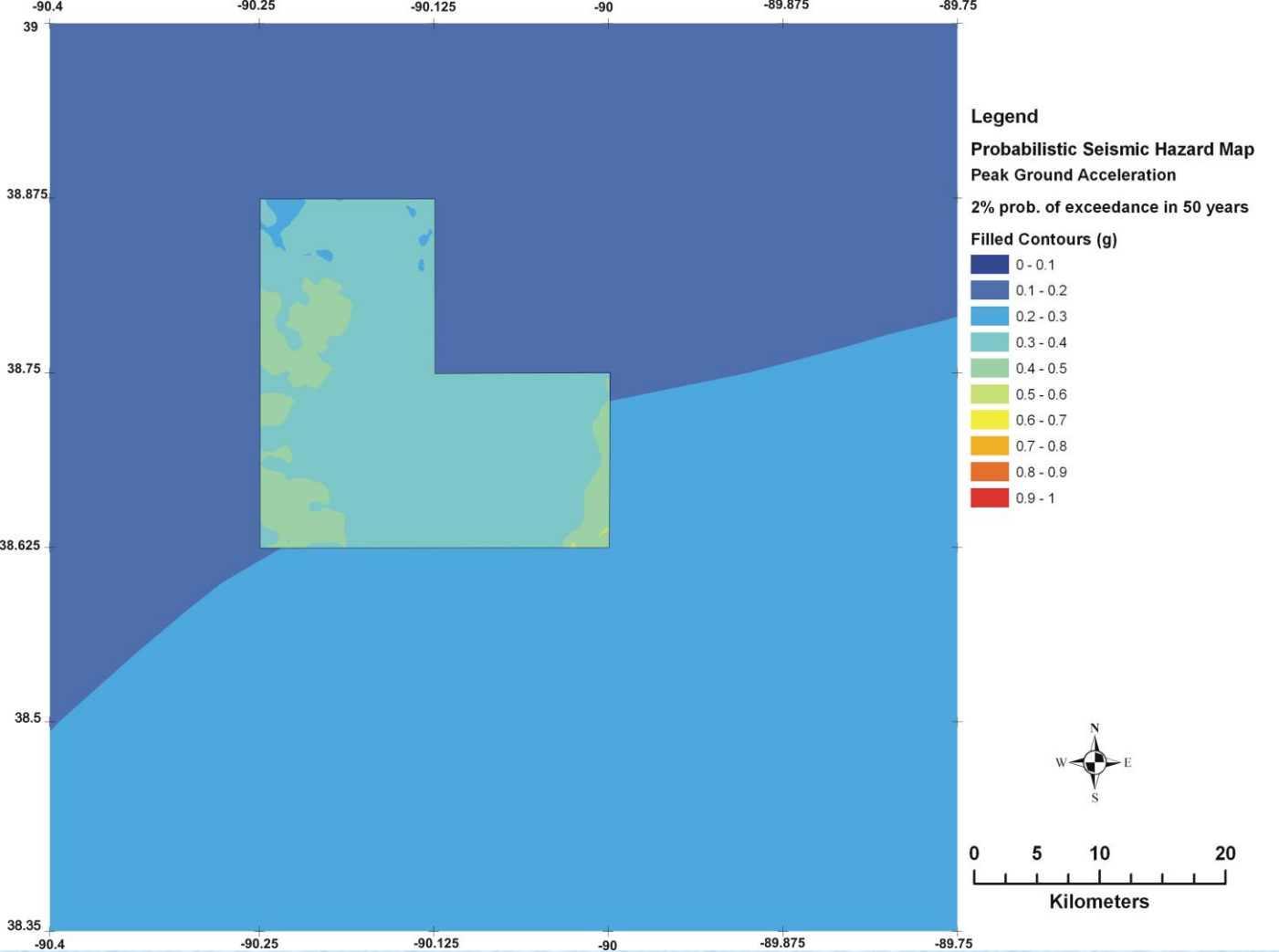
However, they do NOT include the effects of local soils, or so-called “site effects”

Urban Seismic Hazard Maps (Memphis and St Louis)

**These include the effects of
variations in local geology**

**Are completely consistent with the
national maps**

**The scale is useful locally, but not
intended to be site-specific**

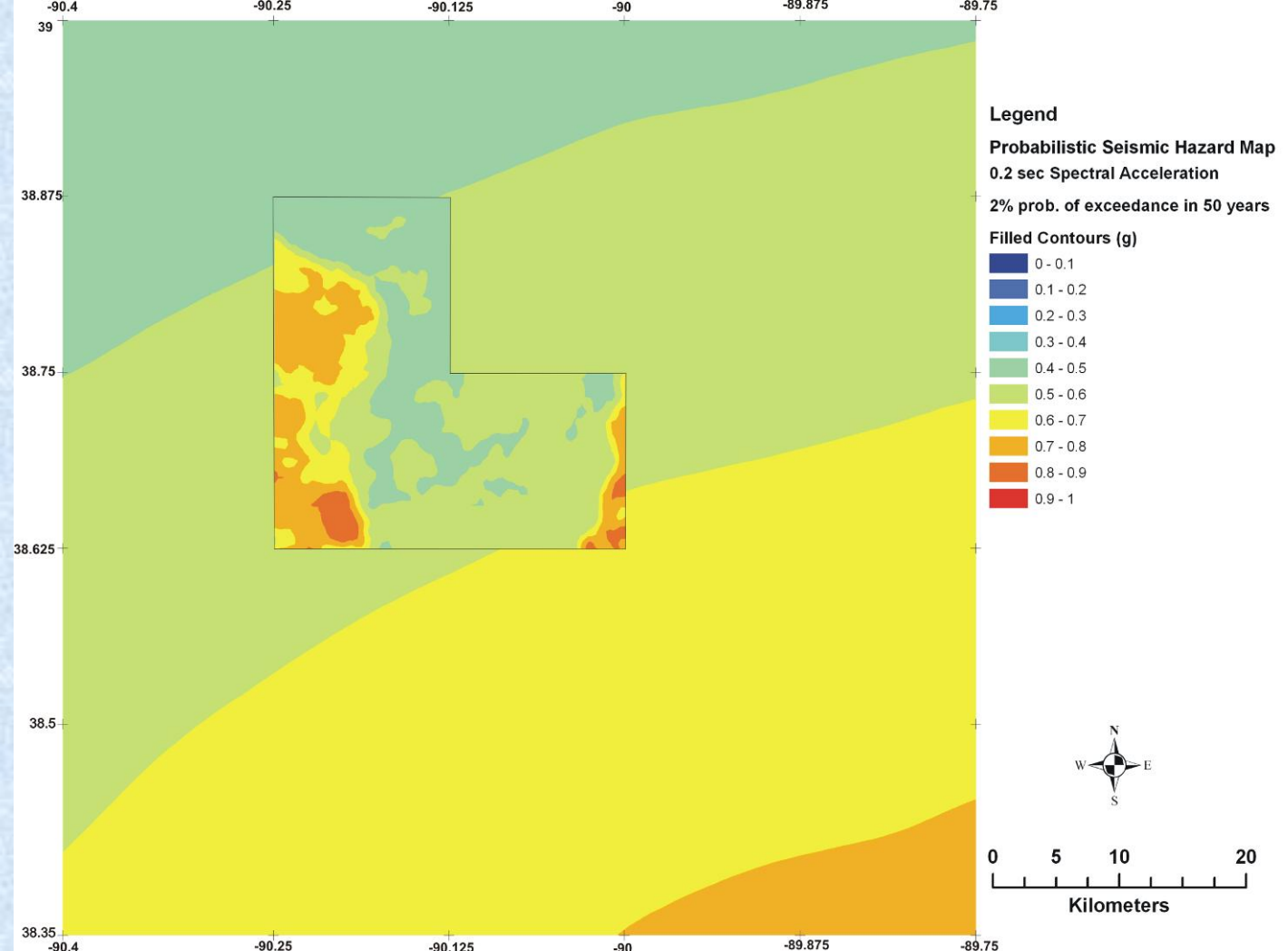


Missouri S&T study study vs USGS National Map (2002)

As much as **300%** greater accelerations in loess
 As much as **200%** greater accelerations in
 alluvium

PGA (g)		Alluvium	Loess
2%-in-50	Max	0.383	0.547
	Min	0.267	0.245
	Mean	0.333	0.423

Missouri S&T study vs USGS National Map (2002)



As much as **200%** greater accelerations in loess
 As much as **20% lower** accelerations in alluvium,
 locally.

0.2 sec SA		Alluvium	Loess
2%-in-50	Max	0.783	0.965
	Min	0.407	0.422
	Mean	0.511	0.750

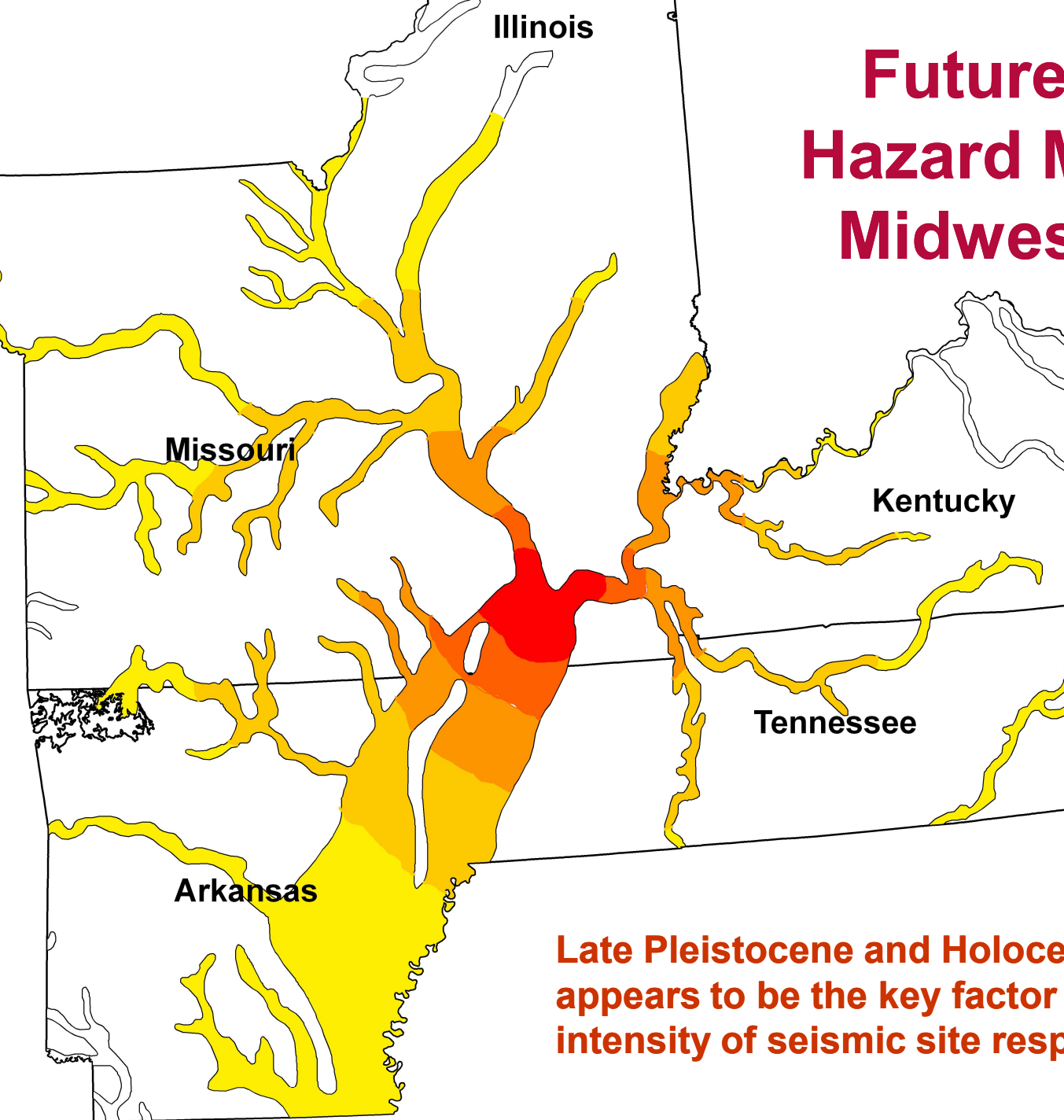
Summary: Shaking intensity is controlled by the underlying geology

2% probability of exceedance in 50 years acceleration values for **loess at 0.2 sec Sa** and for **alluvium at 1 sec Sa** values appear to be large enough to cause structural damage in the St. Louis Metro Area.

Earthquake forces may be most severe for short period structures, on upland sites underlain by loess.

Earthquake forces may also be severe for long period structures on deep (>14 m) alluvial sites, in the natural flood plains.

Future Seismic Hazard Map for the Midwestern USA



Late Pleistocene and Holocene Alluvial thickness appears to be the key factor in controlling local intensity of seismic site response

Future Work - 1

The results indicate that the site amplification on alluvial sites is most influenced by the **unit thickness**. Therefore, more data is needed to better define the variations of thickness in alluvium.

The **depth to top-of-bedrock** (soil cap thickness) map was prepared using kriging methods. There are inherent advantages and disadvantages associated with this methodology. Every effort should be made to amend this map with additional data and hand-estimate the bedrock topography, in lieu of kriging, to elicit a more accurate prediction (ignoring 3D effects).

Future Work - 2

Site amplification and seismic hazard depend largely on the estimated input parameters.

Some of these parameters must be estimated more accurately, i.e., maps showing thickness of the soil cap.

The hazard results are based on the 2002 USGS model. The USGS has since updated their models with a new National Map in 2008. New calculations need to be performed to evaluate how these changes compare with the estimates in the Missouri S&T study.

Acknowledgments

Grants from the U.S. Geological Survey-National Earthquake Hazard Reduction Program and the USGS Central-Eastern U.S. office (Rob Williams and Eugene Schweig)

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**St. Louis Area Earthquake Hazard Mapping Program
Technical Working Group and**

Phyllis Steckel, RG – SLAEHMP TWG facilitator

**Dr. Deniz Karadeniz, Dr. Jae-won Chung, Ece Karadeniz, and
David J. Hoffman of the Missouri S&T research team**

This lecture will be posted at:

<http://web.mst.edu/~rogersda/nmsz/>