

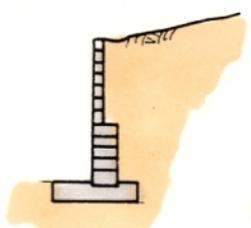
## Part 4

# CANTILEVER WALLS

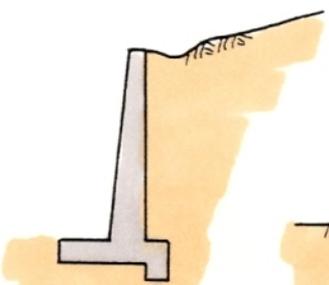
# CANTILEVER WALLS

- **Cantilever theory** was introduced by Galileo in the 16<sup>th</sup> Century, then advanced by Sir John Fowler and Sir Benjamin Baker in the 19<sup>th</sup> Century
- **Reinforced concrete retaining walls** were introduced by the Chicago, Burlington and Quincy Railroad in the 1880s

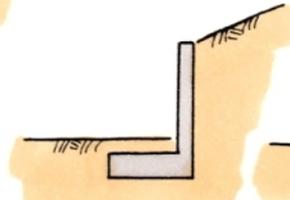




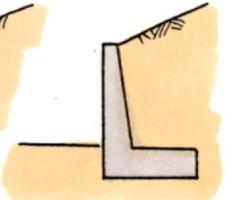
Masonry Block  
or Speed Block



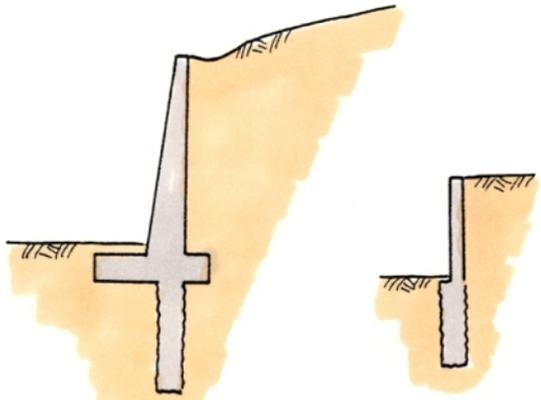
reinforced  
concrete  
cantilever



inside stem  
wall



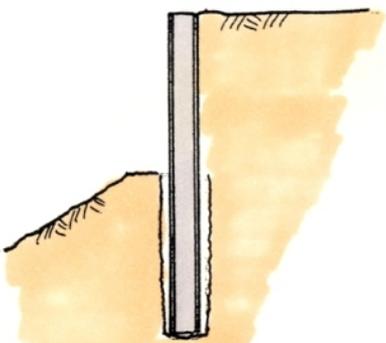
reverse stem  
wall



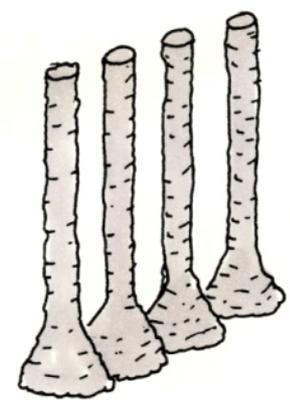
pier supported reinforced  
concrete walls



cast-in-place  
reinforced  
concrete piers  
with inter-  
connecting  
grade beam

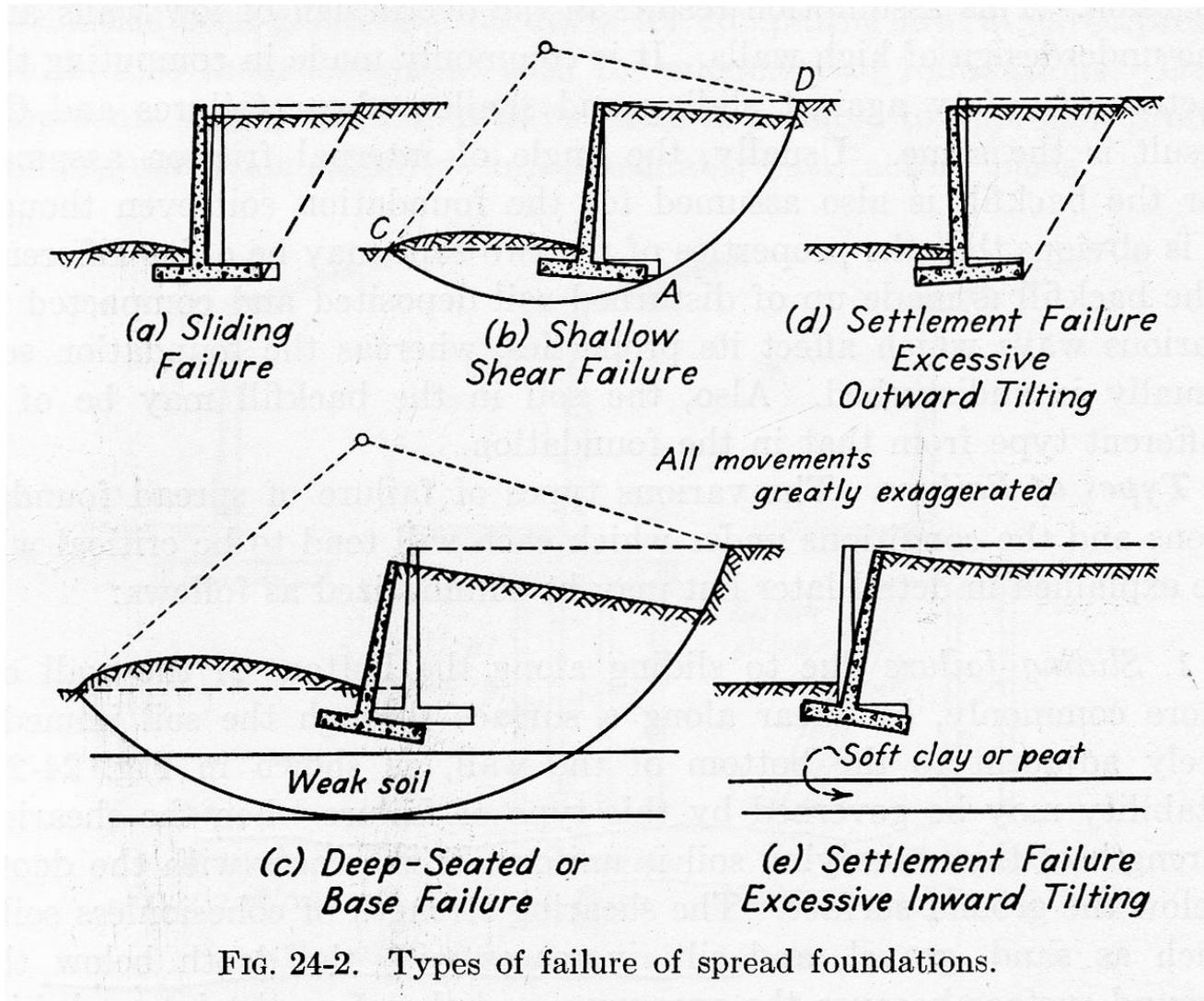


steel H-pile wall



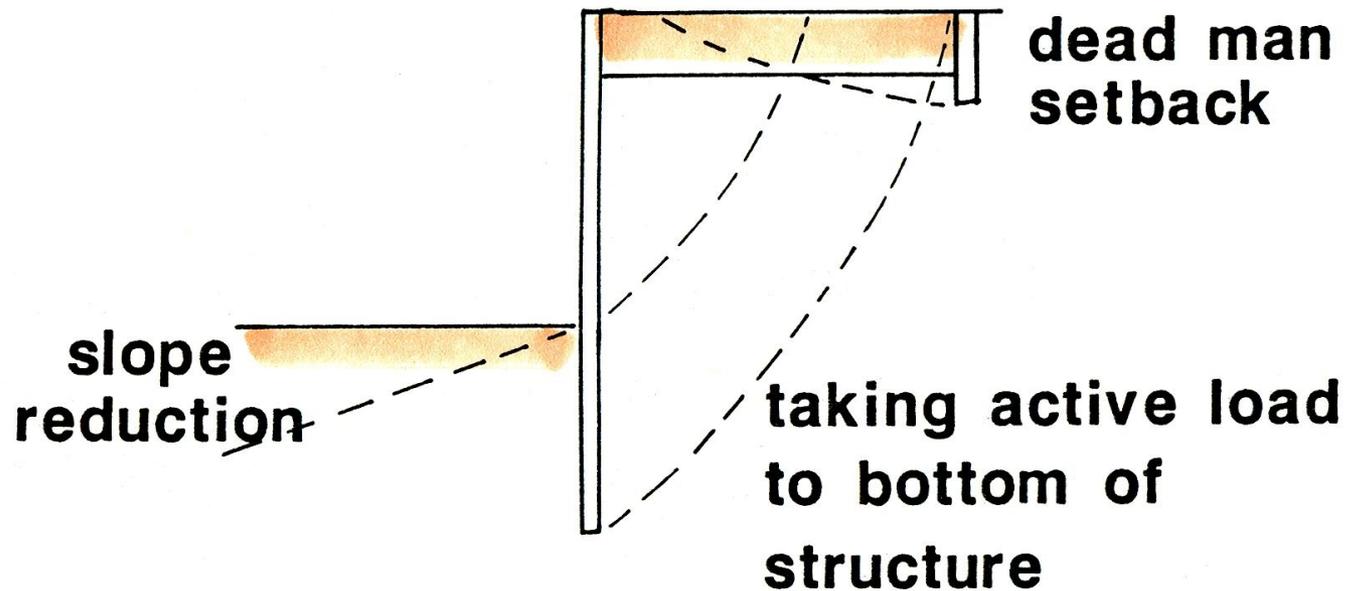
cast-in-place  
caissons with  
interconnecting  
underream cones

- **Common types of cantilever retaining wall systems**
- **Pile driving dates back to the time of the Romans**
- **Large diameter augers allow structures to extend into any kind of material**

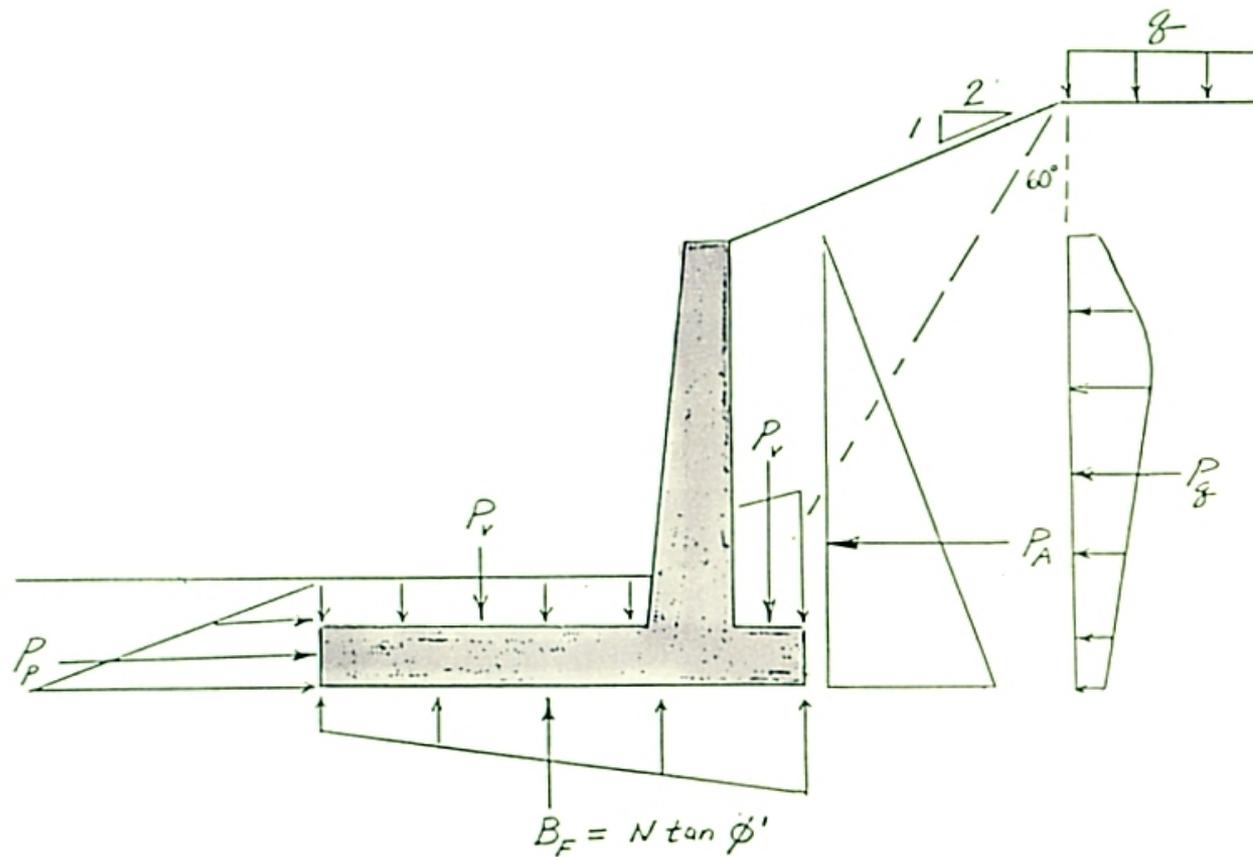


- Common failure modes of cantilever retaining walls, from Huntington's *Earth Pressures and Retaining Walls* (1957)

# CANTILEVER WALLS

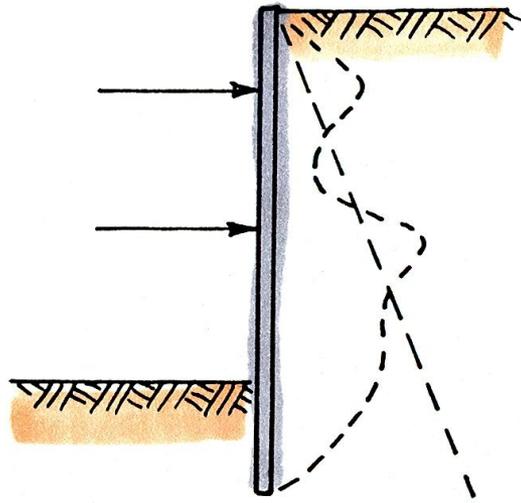


- The basic design precepts employed in cantilever walls include considerations of dead man tiebacks, taking active pressures to the bottom of the wall and considering any reductions in passive resistance for inclined slopes. **Deflection generally governs design for cantilever walls more than 18 feet high.**

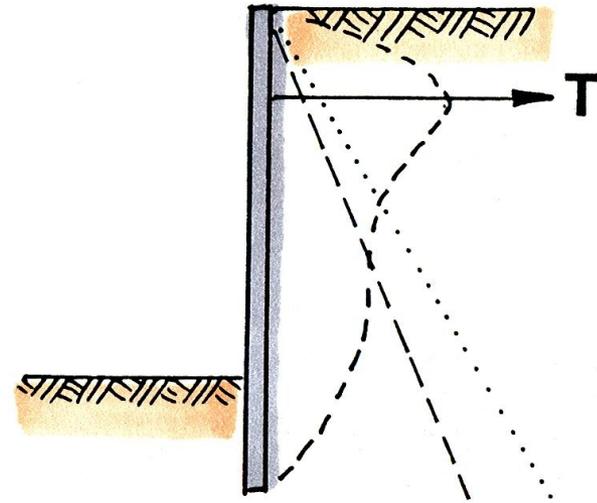


- **Design components** for conventional cantilever walls on spread footings.
- The **resultant thrust** should project through the middle third of the footing or eccentric loading may result in localized bearing failure.

## ACTUAL LOAD DISTRIBUTIONS



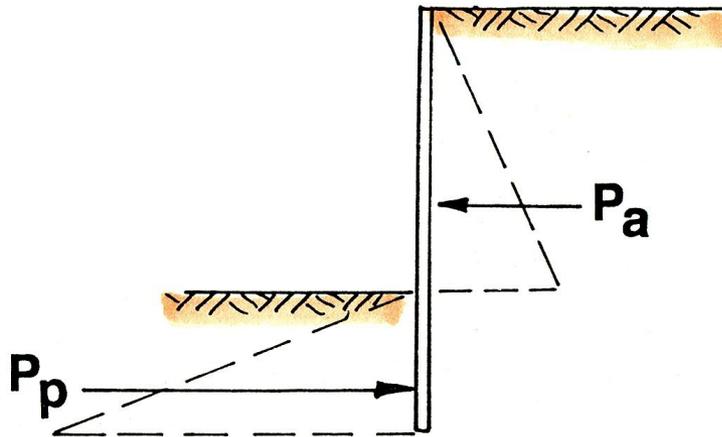
internally-braced  
excavation



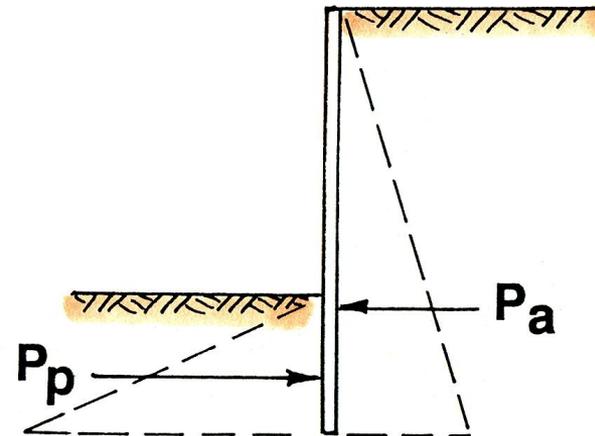
Tie-backs

- The **Rankine linear approximation** tends to under and overestimate actual loads, as sketched above.
- The period of relaxation between excavation and placement of struts also affects measured soils loads, sketched at left.

# COMMON ERRORS



WRONG



CORRECT

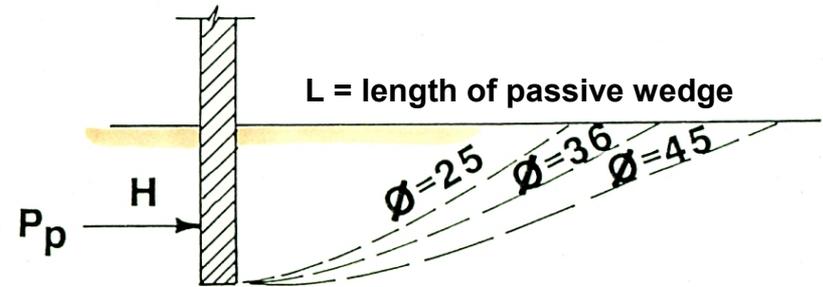
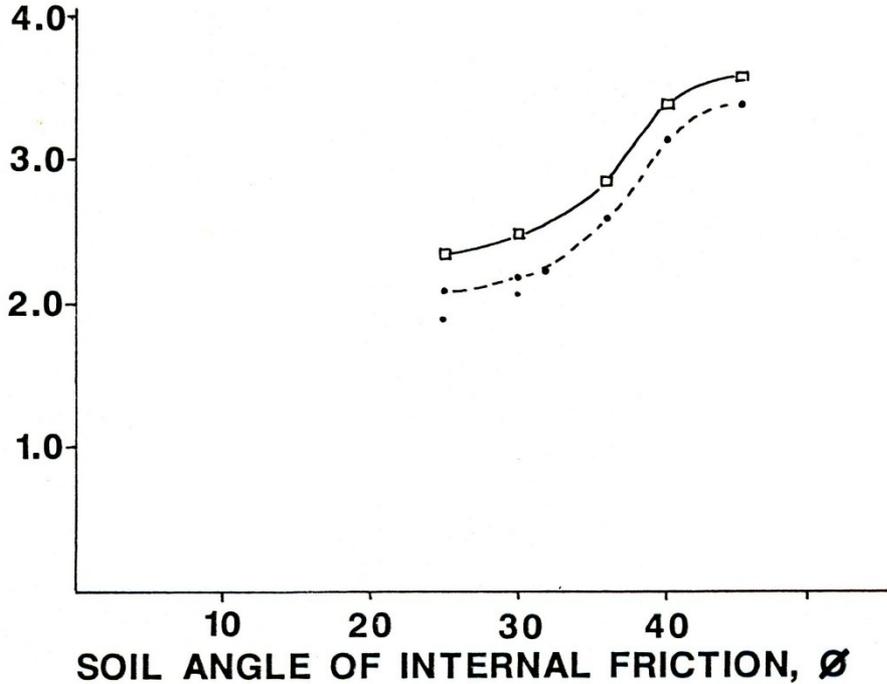
- A common error in design of cantilever walls is to ignore the active pressure load below the level of embedment, shown at left.
- **The Rankine pressure distribution tends to become more valid with time**, as the mobilized soils relax and reach equilibrium for the imposed loads.

# COMMON ERRORS



- **This wall was designed to repair a roadside slip-out. It utilized a slope correction factor for passive resistance, but ignored the possibility of the downslope moving away from the caissons, which it did sometime later.**

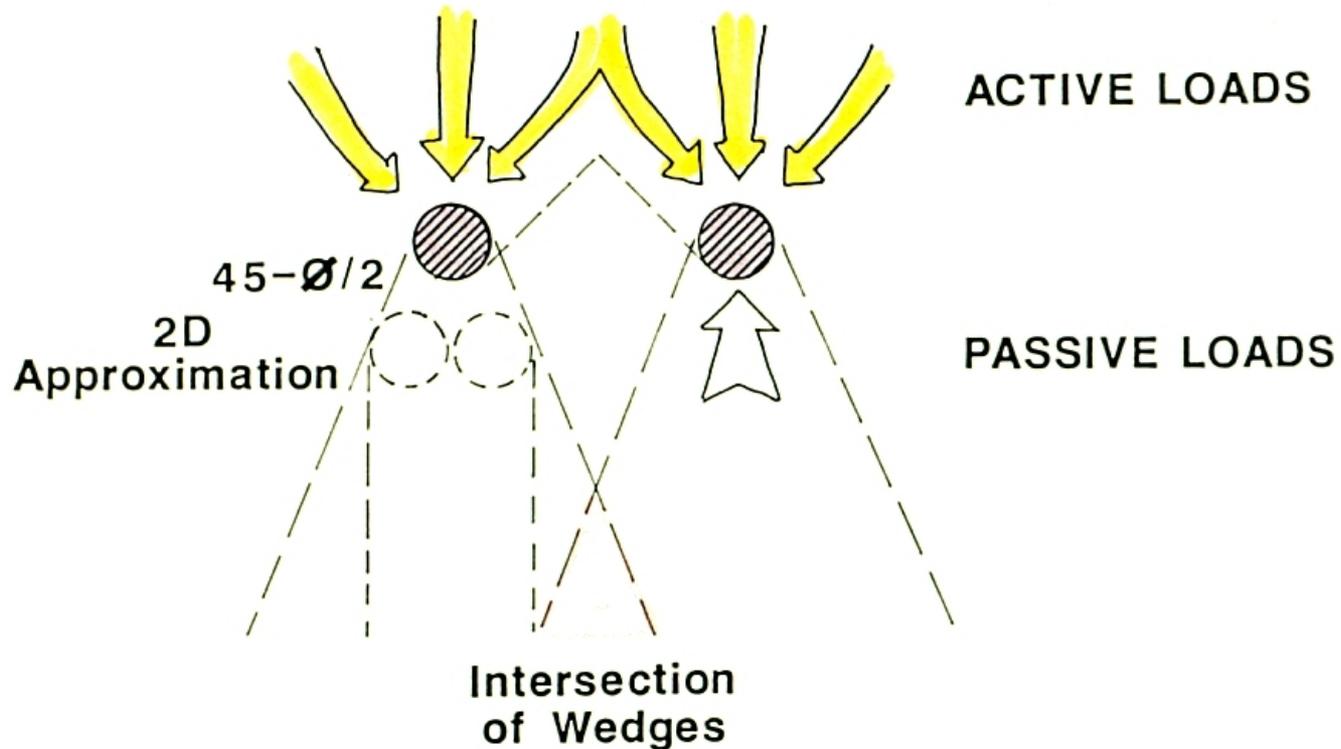
RATIO OF L/H for Passive Soil Wedge



**Ratio of length-to-height for passive soil resistance wedges on resisting side of cantilever elements, as a function of soil friction,  $\phi$ . Note that the normal range is something between 2 and 4.**

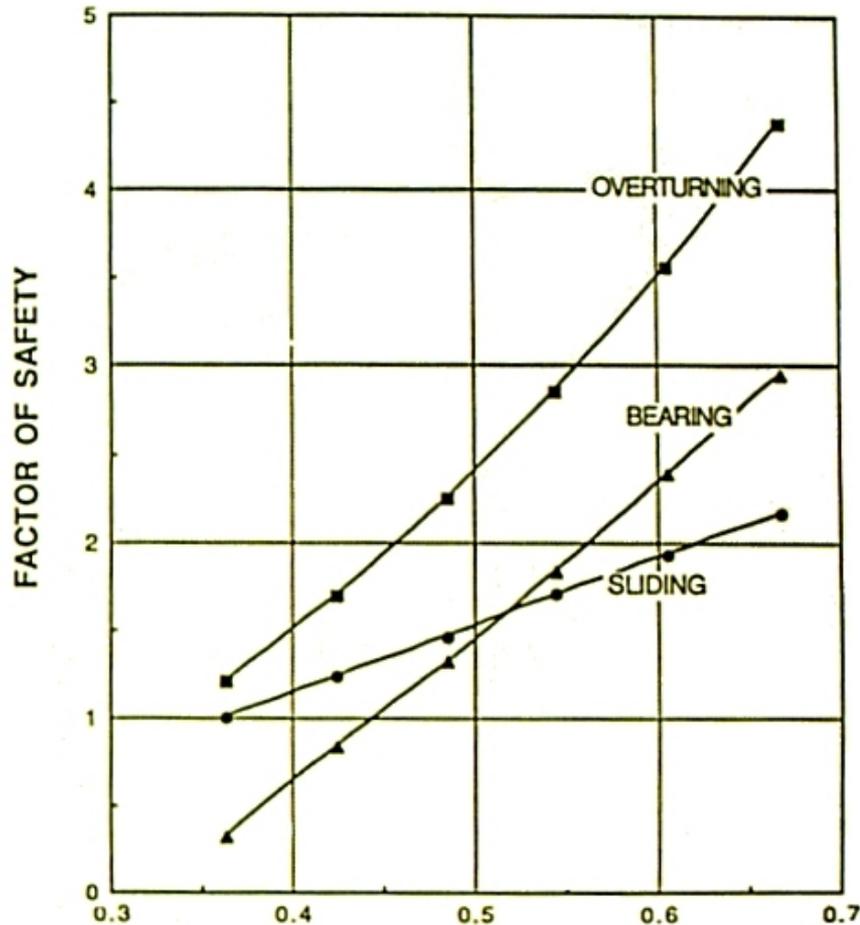


- **When caissons are spaced 5 pier diameters or less apart, ground loads will tend to arch onto the stiff inclusions, as shown here.**
- **A semi-circular zone of tension will develop between the piers.**



- If piers are within  $5D$  of each other active pressures should be carried over a tributary area equal to the center-to-center spacing of the piers.
- The passive pressure should be limited to a width of two pier diameters.

EXTERNAL STABILITY OF CANTILEVER WALL  
(NEGLECTING PASSIVE PRESSURE @ TOE)



- Factor of safety versus height-to-width ratio of conventional cantilever retaining walls, neglecting passive pressure
- As the footing width is increased the FS against overturning, bearing capacity and base sliding all increase